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## CCMC Report Number 13300-R









High Performance Floor & Roof Systems











WSG CANADA, March 2012

# The SIMPLE FRAMING SYSTEM®





### It's the SIMPLE FRAMING SYSTEM®.

featuring beams, joists and rim boards that work together as a system, so you spend less time cutting and fitting. In fact, the SIMPLE FRAMING SYSTEM® uses fewer pieces and longer lengths than conventional framing, so you'll complete jobs in less time.

### You'll Build Better Homes with the SIMPLE FRAMING SYSTEM®

Now it's easier than ever to design and build better floor systems. When you specify the SIMPLE FRAMING SYSTEM®, your clients will have fewer problems with squeaky floors and ceiling gypsum board cracks. The SIMPLE FRAMING SYSTEM® also means overall better floor and roof framing than dimension lumber allows.

### Better Framing Doesn't Have to Cost More

Boise Cascade Engineered Wood Products' SIMPLE FRAMING SYSTEM® often costs less than conventional framing methods when

the resulting reduced labor and materials waste are considered. There's less sorting and cost associated with disposing of waste because you order only what you need. Although our longer lengths help your clients get the job done faster, they cost no more.

### **Environmentally Sound**

As an added bonus, floor and roof systems built with BCI® Joists require about half the number of trees as those built with dimension lumber. This helps you design a home both you and future generations will be proud to own.

### What Makes the SIMPLE FRAMING SYSTEM® So Simple?

### ☑ Floor and Roof Framing with **BCI®** Joists

Light in weight, but heavy-duty, BCI® Joists have a better strength / weight ratio than dimension lumber. Knockouts can be removed for cross-ventilation and wiring.

### Ceilings Framed with BCI<sup>®</sup> Joists

The consistent size of BCI® Joists helps keep gypsum board flat and free of unsightly nail pops and ugly shadows, while keeping finish work to a minimum.

### ☑ VERSA-LAM<sup>®</sup> Beams for Floor and Roof Framing

These highly-stable beams are free of the large-scale defects that plague dimension beams. The result is quieter, flatter floors (no camber) and no shrinkage-related call-backs.

### Boise Cascade Rimboard

Boise Cascade Engineered Wood Products offer several engineered rimboard products regionally, including BC RIM BOARD® OSB, VERSA-RIM<sup>®</sup>, VERSA-STRAND™ 0.8 and VERSA-LAM® 1.4 1800 (check supplier or Boise Cascade EWP representative for availability). These products work with BCI® Joists to provide a solid connection at the critical floor/wall intersection.

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# **BCI®** Joist Architectural Specifications

**Scope:** This work includes the complete furnishing and installation of all BCI<sup>®</sup> Joists as shown on the drawings, herein specified and necessary to complete the work.

**Materials:** BCI<sup>®</sup> Joists shall be manufactured by Boise Cascade Engineered Wood Products with oriented strand board webs, VERSA-LAM<sup>®</sup> laminated veneer lumber flanges and waterproof, structural adhesives.

Joist webs shall be graded Structural I Exposure 1 by an agency listed by a model code evaluation service.

Strands on the face layers of the web panels shall be oriented vertically in the joist. The web panels shall be glued together to form a continuous web member. The web panels shall be machined to fit into a groove in the center of the wide face of the flange members so as to form a pressed glue joint at that junction.

**Design:** The BCI<sup>®</sup> Joists shall be sized and detailed to fit the dimensions and loads indicated on the plans. All designs shall be in accordance with allowable values and section properties developed in accordance with ASTM D5055, CSA O86, and listed under a CCMC product evaluation.

**Drawing:** Additional drawings showing layout and detail necessary for determining fit and placement in the building are (are not) to be provided by the supplier.

**Fabrication:** The BCI<sup>®</sup> Joists and section properties shall be manufactured in a plant evaluated for fabrication by the governing code evaluation service and under the supervision of a third-party inspection agency listed by the corresponding evaluation service.

**Storage and Installation:** The BCI<sup>®</sup> Joists, if stored prior to erection, shall be stored in a vertical and level position and protected from the weather. They shall be handled with care so they are not damaged.

The BCI® Joists are to be installed in accordance with the plans and the Boise Cascade Engineered Wood Products Installation Guide. Temporary construction loads which cause stresses beyond design limits are not permitted. Erection bracing shall be provided to keep the BCI® Joists straight and plumb as

required and to assure adequate lateral support for the individual BCI<sup>®</sup> Joists and the entire system until the sheathing material has been applied.

**Codes:** The BCI<sup>®</sup> Joists shall be evaluated by the CCMC evaluation service.

## Lifetime Guaranteed Quality and Performance

Boise Cascade warrants its BCI<sup>®</sup> Joist, VERSA-LAM<sup>®</sup>, and ALLJOIST<sup>®</sup> products to comply with our specifications, to be free from defects in material and workmanship, and to meet or exceed our performance specifications for the normal and expected life of the structure when correctly stored, installed and used according to our Installation Guide.

### For information about

Boise Cascade's engineered wood products, including sales terms and conditions, warranties and disclaimers, visit our website at www.BC.com/ewp

### BOISE CASCADE IS CERTIFIED BY SFI, AMERICA'S LEADING FORESTRY CERTIFICATION PROGRAM:

Boise Cascade doesn't own forests, but buys wood fiber in compliance with SFI, the Sustainable Forestry Initiative<sup>®</sup>, which certifies the dominant share of North American forest acreage -- 160+ million acres. Boise Cascade is an SFI chain-of-custody certified national supplier. Chain-of-custody tracks and records possession and transfer of wood fiber from forest

of origin through all stages of distribution and production to the homebuilder. Chain-ofcustody assures that Boise Cascade products are made using fiber from responsibly-



managed forests and not from areas that are illegally harvested, major tropical wilderness areas or biodiversity hotspots. Boise Cascade's computerized chain-of-custody system documents sourcing of all wood fiber purchased, ensuring that none gets into Boise Cascade inventory unless it comes from acceptable sources.

# Western Product Profiles



# **Factored Resistances**



							Factored E Resistar	nd Bearing nce (lbs)	Factored Intern Resistar	nediate Bearing nce (lbs)
		Factored	Factored		Shear		1½" Min. Bea	ring Length <sup>(2)</sup>	31⁄2" Min. Be	aring Length
BCI® Joist	Joist Depth	Moment Resistance	Shear Resistance	Joist Stiffness El	Deformation Coefficient, K	Joist Weight	No Web Stiffeners	WITH Web Stiffeners	No Web Stiffeners	WITH Web Stiffeners
Series	[in]	[lbs-ft]	[lbs]	[x10 <sup>6</sup> lbs-in <sup>2</sup> ]	[x10 <sup>6</sup> lbs]	[lbs/ft]	[lbs]	[lbs]	[lbs]	[lbs]
	91⁄2	4130	2520	160	5.2	2.0	1530	2360	3360	4350
5000 1.7	111⁄8	5300	2770	265	6.4	2.3	1530	2410	3570	4630
	14	6280	2990	390	7.6	2.5	1530	2530	3760	4890
	91⁄2	5310	2730	190	5.2	2.2	1920	2340	3890	4960
6000	111⁄8	6810	3050	320	6.5	2.5	1930	2640	4020	5310
1.8	14	8080	3330	470	7.6	2.7	1940	2900	4140	5630
	16	9220	3600	635	8.7	2.9	1950	3140	4250	5920
	91⁄2	5880	2730	210	5.3	2.3	1920	2340	3890	4960
6500	111⁄8	7540	3050	350	6.5	2.6	1930	2640	4020	5310
1.8	14	8950	3330	515	7.7	2.9	1940	2900	4140	5630
	16	10210	3600	690	8.7	3.1	1950	3140	4250	5920
	111⁄8	10370	3050	430	6.6	2.9	1930	2640	4020	5310
60 2.0	14	12370	3330	635	7.7	3.1	1940	2900	4140	5630
	16	14170	3600	860	8.7	3.3	1950	3140	4250	5920
	111⁄8	15870	3130	645	6.7	3.9	2000	3160	4850	5910
	14	18940	3640	940	7.8	4.1	2040	3640	4870	6370
90 2.0	16	21700	4120	1275	8.9	4.4	2080	4080	4900	6790
	18 <sup>(3)</sup>	24430	4600	1660	10	4.6	N/A	4530	N/A	7220
	20(3)	27130	5080	2100	11.1	4.8	N/A	4970	N/A	7640

#### NOTES:

(1) All resistance factors, as per CSA O86 have been applied.

(2) Minimum bearing length at end support is 1½" for BCI<sup>®</sup> 5000, 6000 and 6500, and 1¾" for BCI<sup>®</sup> 60 and 90.

(3) BCI® Joists deeper than 16" require web stiffeners at all bearing locations

(4) The BCI<sup>®</sup> Joist deflection under uniform load may be calculated with the equation to the right:

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$$\Delta = \frac{5wl^4}{384 EI} + \frac{wl^2}{K}$$

 $\Delta$  = deflection [in]

- w = uniform load [lb/in]
- l = clear span [in]
- *EI* = bending stiffness [lb-in<sup>2</sup>]
- *K* = shear deformation coefficient [lb]

## About Floor Performance

Homeowner's expectations and opinions vary greatly due to the subjective nature of rating a new floor. Communication with the ultimate end user to determine their expectation is critical. *Vibration* is usually the cause of most complaints. Installing lateral bridging may help; however, squeaks may occur if not installed properly. Spacing the joists closer together does little to affect the perception of the

floor's performance. The most common methods used to increase the performance and reduce vibration of wood floor systems is to *increase the joist depth, limit joist deflections, glue and screw a thicker tongue-and-groove subfloor, install the joists vertically plumb with level-bearing supports, and install a direct-attached ceiling to the bottom flange of the joists.* 

	5⁄₃" Subfloor (Nailed)												
Live Load	d: 40 psf			Simple	e Span					Continuo	ous Span		
Dead Load: 15 psf Bare Joist					Applied 3	∕₂" Gypsur	n Ceiling		Bare Joist		Applied 1/2" Gypsum Ceiling		
Joist Series	Depth [in]	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"
BCI®	91⁄2	14'-2"	13'-3"	12'-8"	14'-7"	13'-7"	13'-1"	15'-4"	14'-4"	13'-9"	15'-10"	14'-9"	14'-2"
5000	111⁄8	15'-11"	14'-10"	14'-3"	16'-5"	15'-4"	14'-8"	17'-3"	16'-1"	15'-6"	17'-9"	16'-7"	15'-11"
1.7	14	17'-4"	16'-2"	15'-6"	17'-10"	16'-8"	16'-0"	19'-0"	17'-6"	16'-10"	19'-9"	18'-1"	17'-5"
DOI®	91⁄2	14'-9"	13'-9"	13'-3"	15'-2"	14'-2"	13'-7"	16'-0"	14'-11"	14'-4"	16'-5"	15'-4"	14'-9"
BCI <sub>®</sub>	111⁄8	16'-7"	15'-5"	14'-10"	17'-0"	15'-10"	15'-3"	17'-11"	16'-9"	16'-1"	18'-7"	17'-3"	16'-6"
18	14	18'-0"	16'-10"	16'-2"	18'-7"	17'-3"	16'-7"	19'-11"	18'-3"	17'-6"	20'-8"	18'-11"	18'-0"
	16	19'-8"	18'-0"	17'-3"	20'-4"	18'-8"	17'-9"	21'-9"	19'-11"	18'-11"	22'-7"	20'-8"	19'-8"
DOI®	91⁄2	15'-1"	14'-1"	13'-6"	15'-5"	14'-5"	13'-10"	16'-4"	15'-3"	14'-7"	16'-9"	15'-8"	15'-0"
BCI®	111⁄8	16'-11"	15'-9"	15'-1"	17'-4"	16'-2"	15'-6"	18'-4"	17'-1"	16'-5"	19'-0"	17'-7"	16'-10"
1.8	14	18'-5"	17'-2"	16'-5"	19'-1"	17'-7"	16'-11"	20'-5"	18'-9"	17'-10"	21'-2"	19'-5"	18'-5"
	16	20'-1"	18'-5"	17'-7"	20'-10"	19'-1"	18'-1"	22'-3"	20'-5"	19'-5"	23'-1"	21'-2"	20'-1"
BCI®	111⁄8	17'-7"	16'-5"	15'-9"	18'-0"	16'-10"	16'-2"	19'-5"	17'-10"	17'-1"	20'-0"	18'-4"	17'-6"
60	14	19'-6"	17'-11"	17'-2"	20'-1"	18'-5"	17'-7"	21'-7"	19'-9"	18'-9"	22'-4"	20'-5"	19'-5"
2.0	16	21'-3"	19'-5"	18'-5"	21'-11"	20'-1"	19'-1"	23'-7"	21'-7"	20'-6"	24'-4"	22'-4"	21'-2"
	111⁄8	19'-6"	17'-10"	17'-1"	20'-0"	18'-3"	17'-6"	21'-7"	19'-9"	18'-9"	22'-3"	20'-4"	19'-3"
BCI®	14	21'-8"	19'-10"	18'-9"	22'-3"	20'-4"	19'-4"	24'-1"	22'-0"	20'-11"	24'-9"	22'-8"	21'-6"
90	16	23'-8"	21'-7"	20'-5"	24'-3"	22'-2"	21'-0"	26'-3"	24'-0"	22'-9"	27'-0"	24'-8"	23'-5"
2.0	18	25'-6"	23'-3"	22'-1"	26'-2"	24'-0"	22'-8"	28'-4"	25'-10"	24'-6"	29'-1"	26'-8"	25'-3"
	20	27'-3"	24'-11"	23'-7"	28'-0"	25'-8"	24'-4"	30'-3"	27'-8"	26'-3"	31'-2"	28'-6"	27'-1"

<sup>3</sup> / <sub>4</sub> " Subfloor (Nailed)														
Live Loa	d: 40 psf			Simple	e Span			Continuous Span						
Dead Loa	ad: 15 psf	Bare Joist Applied ½" Gypsum Ceiling						Bare Joist Applied ½" Gypsum Ceiling						
Joist Series	Depth [in]	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	
BCI®	91⁄2	14'-10"	13'-10"	13'-3"	15'-2"	14'-2"	13'-7"	16'-0"	15'-0"	14'-4"	16'-6"	15'-4"	14'-9"	
5000	111/8	16'-8"	15'-6"	14'-10"	17'-1"	15'-11"	15'-3"	18'-0"	16'-10"	16'-1"	18'-8"	17'-4"	16'-7"	
1.7	14	18'-2"	16'-11"	16'-2"	18'-9"	17'-5"	16'-8"	20'-1"	18'-5"	17'-6"	20'-9"	19'-1"	18'-1"	
DOI®	91⁄2	15'-5"	14'-5"	13'-9"	15'-9"	14'-9"	14'-1"	16'-8"	15'-7"	14'-11"	17'-1"	16'-0"	15'-4"	
BCI®	111/8	17'-4"	16'-2"	15'-5"	17'-9"	16'-6"	15'-10"	18'-11"	17'-6"	16'-9"	19'-6"	17'-11"	17'-2"	
1.8	14	19'-0"	17'-7"	16'-10"	19'-7"	18'-0"	17'-3"	21'-1"	19'-3"	18'-3"	21'-9"	20'-0"	18'-11"	
	16	20'-9"	19'-0"	18'-0"	21'-5"	19'-8"	18'-7"	23'-0"	21'-1"	19'-11"	23'-9"	21'-10"	20'-8"	
DOI®	91⁄2	15'-9"	14'-8"	14'-1"	16'-1"	15'-0"	14'-4"	17'-0"	15'-11"	15'-3"	17'-5"	16'-3"	15'-7"	
BCI®	111/8	17'-8"	16'-5"	15'-9"	18'-1"	16'-10"	16'-1"	19'-5"	17'-10"	17'-1"	20'-0"	18'-4"	17'-6"	
1.8	14	19'-6"	17'-11"	17'-2"	20'-1"	18'-5"	17'-7"	21'-7"	19'-9"	18'-9"	22'-3"	20'-5"	19'-4"	
	16	21'-3"	19'-5"	18'-5"	21'-11"	20'-1"	19'-0"	23'-7"	21'-7"	20'-5"	24'-3"	22'-4"	21'-1"	
BCI®	111⁄8	18'-6"	17'-2"	16'-5"	19'-0"	17'-6"	16'-9"	20'-6"	18'-9"	17'-10"	21'-1"	19'-4"	18'-3"	
60	14	20'-8"	18'-10"	17'-11"	21'-2"	19'-5"	18'-4"	22'-10"	20'-11"	19'-9"	23'-6"	21'-7"	20'-5"	
2.0	16	22'-6"	20'-7"	19'-5"	23'-1"	21'-2"	20'-0"	24'-11"	22'-10"	21'-7"	25'-8"	23'-6"	22'-3"	
	111/8	20'-8"	18'-10"	17'-10"	21'-1"	19'-3"	18'-2"	22'-10"	20'-11"	19'-9"	23'-5"	21'-5"	20'-3"	
BCI®	14	23'-0"	20'-11"	19'-9"	23'-6"	21'-6"	20'-3"	25'-6"	23'-3"	22'-0"	26'-1"	23'-10"	22'-7"	
90	16	25'-0"	22'-10"	21'-7"	25'-7"	23'-5"	22'-1"	27'-9"	25'-5"	24'-0"	28'-5"	26'-0"	24'-7"	
2.0	18	27'-0"	24'-8"	23'-3"	27'-7"	25'-3"	23'-11"	29'-11"	27'-5"	25'-10"	30'-7"	28'-1"	26'-7"	
	20	201 11"	26' 4"	24' 11"	20' 6"	27' 0"	25' 7"	221 0"	201 2"	27' 0"	22' 10"	20' 1"	20' 5"	

#### WARNING: Use of Span Tables for Commercial Projects (NBCC2005: Part 4) All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.10. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2005). Table 4.1.5.10 in NBCC (2005) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration.

#### NOTES:

- Tables are based on a uniform 40 psf live load and 15 psf dead load (Standard Term Load Duration).
- Floor tile will increase dead load and may require specific deflection limits.
- Minimum bearing length at end supports is 1½" for BCI® 5000, 6000 and 6500, and 1¾" for BCI® 60 and 90.
- Stiffeners required at ALL bearing locations for joists deeper than 16" (end bearing, interior bearing, and concentrated load locations).
- Maximum spans are measured in between the supports (clearspan) and are based on uniformly loaded joists.
- Live load deflection is limited to L/360 and Total load deflection to L/240. Deflections are based on the bare joist stiffness.
- Spans shown are in accordance with NBCC2005: Part 9, and standard CAN-CSA O86-01.
- When using continuous spans over an intermediate bearing, the shortest span shall not be less than 50% of the longest adjacent span. For other conditions, please contact your distributor or Boise Cascade EWP, for assistance.
- It may be possible to exceed the limitations of these tables by analyzing a specific application with the Boise Cascade BC CALC® software and Boise Cascade WoodSizer software.
- The subfloor shall be CSA rated Oriented Strand Board (OSB), Canadian Softwood Plywood (CSP), or Douglas Fir Plywood (DFP).

## About Floor Performance

Homeowner's expectations and opinions vary greatly due to the subjective nature of rating a new floor. Communication with the ultimate end user to determine their expectation is critical. *Vibration* is usually the cause of most complaints. Installing lateral bridging may help; however, squeaks may occur if not installed properly. Spacing the joists closer together does little to affect the perception of the

floor's performance. The most common methods used to increase the performance and reduce vibration of wood floor systems is to increase the joist depth, limit joist deflections, glue and screw a thicker tongue-and-groove subfloor, install the joists vertically plumb with level-bearing supports, and install a direct-attached ceiling to the bottom flange of the joists.

	5⁄8" Subfloor (Glued & Nailed)													
Live Loa	d: 40 psf			Simple		Continuous Span								
Dead Loa	ad: 15 psf		Bare Joist	:	Applied	½" Gypsur	n Ceiling		Bare Joist	t	Applied 1/2" Gypsum Ceiling			
Joist Series	Depth [in]	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	
5000	91⁄2	15'-2"	14'-4"	13'-11"	15'-8"	14'-10"	14'-4"	16'-5"	15'-6"	15'-0"	17'-0"	16'-1"	15'-7"	
5000	111⁄8	17'-0"	16'-1"	15'-6"	17'-7"	16'-7"	16'-1"	18'-6"	17'-4"	16'-10"	19'-3"	18'-0"	17'-5"	
1.7	14	18'-7"	17'-5"	16'-10"	19'-4"	18'-0"	17'-5"	20'-6"	19'-1"	18'-4"	21'-5"	20'-0"	19'-2"	
	9½	15'-8"	14'-10"	14'-4"	16'-2"	15'-3"	14'-9"	17'-0"	16'-0"	15'-6"	17'-6"	16'-6"	16'-0"	
6000	111⁄8	17'-7"	16'-7"	16'-0"	18'-1"	17'-1"	16'-6"	19'-3"	17'-11"	17'-4"	20'-0"	18'-7"	17'-11"	
1.8	14	19'-4"	18'-0"	17'-4"	20'-1"	18'-8"	17'-11"	21'-4"	19'-10"	19'-0"	22'-3"	20'-8"	19'-10"	
	16	21'-0"	19'-6"	18'-8"	21'-10"	20'-4"	19'-5"	23'-3"	21'-7"	20'-8"	24'-2"	22'-6"	21'-7"	
	9½	16'-0"	15'-1"	14'-7"	16'-5"	15'-6"	14'-11"	17'-3"	16'-4"	15'-9"	17'-9"	16'-9"	16'-3"	
6500	111⁄8	17'-10"	16'-10"	16'-3"	18'-5"	17'-4"	16'-9"	19'-8"	18'-3"	17'-7"	20'-5"	19'-0"	18'-2"	
1.8	14	19'-9"	18'-3"	17'-8"	20'-5"	19'-0"	18'-2"	21'-10"	20'-3"	19'-5"	22'-8"	21'-1"	20'-2"	
	16	21'-5"	19'-10"	19'-0"	22'-3"	20'-8"	19'-9"	23'-9"	22'-0"	21'-1"	24'-8"	22'-11"	22'-0"	
60	111⁄8	18'-7"	17'-5"	16'-10"	19'-2"	17'-10"	17'-3"	20'-7"	19'-1"	18'-3"	21'-3"	19'-9"	18'-11"	
20	14	20'-8"	19'-2"	18'-4"	21'-4"	19'-10"	18'-11"	22'-11"	21'-2"	20'-3"	23'-8"	22'-0"	21'-0"	
2.0	16	22'-6"	20'-10"	19'-11"	23'-3"	21'-7"	20'-8"	24'-11"	23'-1"	22'-1"	25'-9"	23'-11"	22'-11"	
	111⁄8	20'-5"	18'-10"	18'-0"	20'-11"	19'-4"	18'-6"	22'-7"	20'-11"	19'-11"	23'-3"	21'-6"	20'-7"	
00	14	22'-8"	20'-11"	19'-11"	23'-3"	21'-6"	20'-7"	25'-1"	23'-2"	22'-2"	25'-10"	23'-11"	22'-10"	
20	16	24'-8"	22'-9"	21'-8"	25'-4"	23'-5"	22'-4"	27'-4"	25'-3"	24'-1"	28'-1"	26'-0"	24'-10"	
2.0	18	26'-7"	24'-6"	23'-4"	27'-4"	25'-3"	24'-1"	29'-5"	27'-2"	25'-11"	30'-3"	28'-0"	26'-9"	
	20	28'-4"	26'-2"	24'-11"	29'-2"	27'-0"	25'-9"	31'-6"	29'-0"	27'-8"	32'-4"	29'-11"	28'-7"	

	<sup>3</sup> / <sub>4</sub> " Subfloor (Glued & Nailed)												
Live Load	d: 40 psf			Simple	e Span	Continuous Span							
Dead Loa	ıd: 15 psf		Bare Joist	i	Applied	1⁄2" Gypsur	m Ceiling	Bare Joist Applied 1/2" Gypsum Ce					
Joist Series	Depth [in]	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"	12"	16"	19.2"
5000	91⁄2	16'-2"	15'-3"	14'-8"	16'-7"	15'-8"	15'-0"	17'-5"	16'-5"	15'-10"	18'-0"	17'-0"	16'-0"
5000	111⁄%	18'-0"	17'-0"	16'-5"	18'-8"	17'-7"	16'-11"	19'-11"	18'-6"	17'-9"	20'-8"	19'-4"	18'-2"
1.7	14	20'-0"	18'-7"	17'-9"	20'-9"	19'-4"	18'-6"	22'-1"	20'-6"	19'-7"	23'-0"	21'-5"	19'-10"
	91⁄2	16'-8"	15'-8"	15'-2"	17'-1"	16'-2"	15'-7"	18'-0"	17'-0"	16'-4"	18'-7"	17'-6"	16'-10"
6000	111⁄8	18'-9"	17'-6"	16'-11"	19'-4"	18'-0"	17'-5"	20'-8"	19'-2"	18'-4"	21'-5"	20'-0"	19'-1"
1.8	14	20'-9"	19'-3"	18'-4"	21'-6"	20'-0"	19'-1"	22'-11"	21'-3"	20'-4"	23'-10"	22'-2"	21'-2"
	16	22'-7"	20'-11"	20'-0"	23'-5"	21'-9"	20'-9"	25'-0"	23'-2"	22'-1"	25'-11"	24'-2"	23'-1"
	91⁄2	16'-11"	15'-11"	15'-4"	17'-4"	16'-4"	15'-9"	18'-4"	17'-3"	16'-8"	19'-0"	17'-9"	17'-1"
6500	111⁄8	19'-1"	17'-10"	17'-2"	19'-9"	18'-4"	17'-8"	21'-1"	19'-7"	18'-8"	21'-10"	20'-4"	19'-5"
1.8	14	21'-2"	19'-8"	18'-9"	21'-11"	20'-4"	19'-5"	23'-5"	21'-9"	20'-9"	24'-3"	22'-7"	21'-7"
	16	23'-0"	21'-4"	20'-4"	23'-10"	22'-2"	21'-2"	25'-6"	23'-7"	22'-6"	26'-4"	24'-6"	23'-5"
00	111⁄8	20'-0"	18'-6"	17'-9"	20'-7"	19'-1"	18'-2"	22'-1"	20'-6"	19'-6"	22'-9"	21'-2"	20'-2"
60 2 0	14	22'-2"	20'-6"	19'-7"	22'-10"	21'-2"	20'-2"	24'-6"	22'-9"	21'-8"	25'-3"	23'-6"	22'-5"
2.0	16	24'-2"	22'-4"	21'-3"	24'-10"	23'-1"	22'-0"	26'-8"	24'-9"	23'-7"	27'-6"	25'-7"	24'-5"
	111⁄8	21'-10"	20'-2"	19'-3"	22'-4"	20'-8"	19'-8"	24'-2"	22'-4"	21'-4"	24'-9"	22'-11"	21'-10"
00	14	24'-3"	22'-5"	21'-4"	24'-10"	23'-0"	21'-10"	26'-11"	24'-10"	23'-7"	27'-6"	25'-6"	24'-3"
90	16	26'-5"	24'-4"	23'-2"	27'-0"	25'-0"	23'-10"	29'-3"	27'-0"	25'-8"	29'-11"	27'-9"	26'-5"
2.0	18	28'-5"	26'-3"	24'-11"	29'-1"	26'-11"	25'-8"	31'-6"	29'-1"	27'-8"	32'-3"	29'-10"	28'-5"
	20	30'-4"	28'-0"	26'-7"	31'-1"	28'-9"	27'-5"	34'-0"	31'_0"	29'-6"	35'_1"	31'_11"	30'-5"

## WARNING: Use of Span Tables for Commercial Projects (NBCC2005: Part 4)

All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.10. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2005). Table 4.1.5.10 in NBCC (2005) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration.

#### NOTES:

- Tables are based on a uniform 40 psf live load and 15 psf dead load (Standard Term Load Duration).
- Floor tile will increase dead load and may require specific deflection limits.
- Minimum bearing length at end supports is 1½" for BCI® 5000, 6000 and 6500, and 1¾" for BCI® 60 and 90.
- Stiffeners required at ALL bearing locations for joists deeper than 16" (end bearing, interior bearing, and concentrated load locations).
- Maximum spans are measured in between the supports (clearspan) and are based on uniformly loaded joists.
- Live load deflection is limited to L/360 and Total load deflection to L/240. Deflections are based on the bare joist stiffness.
- Spans shown are in accordance with NBCC2005: Part 9, and standard CAN-CSA O86-01.
- When using continuous spans over an intermediate bearing, the shortest span shall not be less than 50% of the longest adjacent span. For other conditions, please contact your distributor or Boise Cascade EWP, for assistance.
- It may be possible to exceed the limitations of these tables by analyzing a specific application with the Boise Cascade BC CALC<sup>®</sup> software and Boise Cascade WoodSizer software.
- The subfloor shall be CSA rated Oriented Strand Board (OSB), Canadian Softwood Plywood (CSP), or Douglas Fir Plywood (DFP).
- Subfloor adhesive shall comply with CGSB standard CAN-CGSB 71.26-M88 "Adhesives for Field-gluing Plywood to Lumber Framing for Floor Systems" or APA Performance Specification AFG-01.

# **Floor Framing**

# **BCI<sup>®</sup> Joists**



BCI® Joists, VERSA-LAM® and ALLJOIST® must be stored, installed and used in accordance with the Boise Cascade EWP Installation Guide, building codes, and to the extent not inconsistent with the Boise Cascade EWP Installation Guide, usual and customary building practices and standards. VERSA-LAM®, ALLJOIST®, and BCI® Joists must be wrapped, covered, and stored off of the ground on stickers at all times prior to installation. VERSA-LAM®, ALLJOIST® and BCI® Joists are intended only

#### SAFETY WARNING

DO NOT ALLOW WORKERS ON BCI® JOISTS UNTIL ALL HANGERS, BCI® RIM JOISTS, RIM BOARDS, BCI® BLOCKING PANELS, X-BRACING AND TEMPORARY 1x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW. SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

- Build a braced end wall at the end of the bay, or permanently install the first eight feet of BCI<sup>®</sup> Joists and the first course of sheathing. As an alternate, temporary sheathing may be nailed to the first four feet of BCI<sup>®</sup> Joists at the end of the bay.
- All hangers, BCI<sup>®</sup> rim joists, rim boards, BCI<sup>®</sup> blocking panels, and x-bracing must be completely installed and properly nailed as each BCI<sup>®</sup> Joist is set.
- Install temporary 1x4 strut lines at no more than eight feet on center as additional BCI<sup>®</sup> Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each BCI<sup>®</sup> Joist with two 2½" (8d) nails.
- The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the BCI<sup>®</sup> Joists to within ½ inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads.
- Do not stack construction materials (sheathing, drywall, etc) in the middle of BCI<sup>®</sup> Joist spans, contact Boise Cascade EWP Engineering for proper storage and shoring information.

for applications that assure no exposure to weather or the elements and an environment that is free from moisture from any source, or any pest, organism or substance which degrades or damages wood or glue bonds. Failure to correctly store, use or install VERSA-LAM®, ALLJOIST®, and BCI® Joist in accordance with the Boise Cascade EWP Installation Guide will void the limited warranty.



# Floor Framing Details





- spacing is 24 inches on center. 14 gauge staples may be substituted for 2½" (8d) nails if the staples penetrate at least 1 inch into the joist.
- Wood screws may be acceptable, contact local building official and/or Boise Cascade EWP Engineering for further information.
- **PROTECT BCI® JOISTS FROM THE WEATHER** BCI® Joists are intended only for applications . that provide permanent protection from the weather. Bundles of BCI<sup>®</sup> Joists should be covered and stored off of the ground on stickers.

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BCI® 5000 rim joist: 2-3" (10d) box nails, one each in the top and bottom flange.

nails, one each in the top and bottom flange.

BCI® 6000, 60 rim joist: 2-31/2" (16d) box

# Web Stiffener Requirements



Web Stiffener Specifications										
Series	For Structral Capacity (Min. Thick)	Lateral Restraint in Hanger	Minimum Width							
5000 1.7	5⁄8"	<sup>3</sup> ⁄4"	2 <sup>5</sup> / <sub>16</sub> "							
6000 1.8	3/4"	7⁄8"	2 <sup>5</sup> /16"							
6500 1.8	3/4"	1" or 11/8"	2 <sup>5</sup> /16"							
60 2.0	3/4"	7⁄8"	2 <sup>5</sup> / <sub>16</sub> "							
90 2.0	2x4 lumber (vertical)									

 Web stiffeners are optional except as noted below.

- Web stiffeners are always required for all 18" and 20" BCI<sup>®</sup> Joist at all bearing locations.
- Web stiffeners are always required in hangers that do not extend up to support the top flange of the BCI<sup>®</sup> Joist. Web stiffeners may be required with certain sloped or skewed hangers or to achieve uplift values. Refer to the hanger manufacturer's installation requirements.
- Web stiffeners may be cut from structural rated wood panels, engineered rimboard or 2x lumber (BCI® 90 only).
- For Structural Capacity: Web stiffeners needed to increase the BCI® Joist's reaction capacity at a specific bearing location.
- Web stiffeners are always required in certain roof applications. See Roof Framing Details on page 13 & 14.
- Web stiffeners are always required under concentrated loads that exceed 1000 pounds. Install the web stiffeners snug to the top flange in this situation. Follow the nailing schedule for intermediate bearings.
- Web stiffeners may be used to increase allowable factored bearing resistances. See Factored Resistances on page 4 or the BC CALC<sup>®</sup> software.

Web	5 Stiffene	r Nailing Sche	edule
BCI <sup>®</sup>	Joist	Bearing	Location
Joist Series	Depth	End	Intermediate
	9½"	2-21⁄2" (8d)	2-21⁄2" (8d)
5000 1.7	111⁄8"	2-21⁄2" (8d)	3-2½" (8d)
	14"	2-21⁄2" (8d)	5-2½" (8d)
	9½"	2-21⁄2" (8d)	2-21⁄2" (8d)
6000 1 9	111⁄8"	2-21⁄2" (8d)	3-21⁄2" (8d)
0000 1.0	14"	2-21⁄2" (8d)	5-2½" (8d)
	16"	2-21⁄2" (8d)	6-2½" (8d)
	91⁄2"	2-21⁄2" (8d)	2-21⁄2" (8d)
6500 1 9	111⁄8"	2-21⁄2" (8d)	3-21⁄2" (8d)
0.1 0000	14"	2-21⁄2" (8d)	5-2½" (8d)
	16"	2-21⁄2" (8d)	6-2½" (8d)
	111⁄8"	2-21⁄2" (8d)	3-2½" (8d)
60 2.0	14"	2-21⁄2" (8d)	5-2½" (8d)
	16"	2-21⁄2" (8d)	6-2½" (8d)
	111⁄8"	3-31⁄2" (16d)	3-31⁄2" (16d)
	14"	5-31⁄2" (16d)	5-31⁄2" (16d)
90 2.0	16"	6-3½" (16d)	6-3½" (16d)
	18"	7-3½" (16d)	7-3½" (16d)
	20"	8-31/2" (16d)	8-31/2" (16d)

# **Connection Details**





# **Reinforced Load Bearing Cantilever Detail**



 The tables and details on pages 10 and 11 indicate the type of reinforcements, if any, that are required for load-bearing cantilevers up to a maximum length of 2'-0". Cantilevers longer than 2'-0" cannot be reinforced. However, longer cantilevers with lower loads may be allowable without reinforcement. Analyze specific applications with the BC CALC<sup>®</sup> software.

#### PLYWOOD / OSB REINFORCEMENT (If Required per Table on page 11)

- <sup>23</sup>/<sub>32</sub>" Min. x 48" long plywood / OSB rated sheathing must match the full depth of the BCI<sup>®</sup> Joist. Nail to the BCI<sup>®</sup> Joist with 2½" (8d) nails at 6" o.c. and nail with 4-2½" (8d) nails into backer block. When reinforcing both sides, stagger nails to limit splitting. Install with horizontal face grain.
- These requirements assume a 100 PLF wall load and apply to BCI<sup>®</sup> 5000 1.7, 6000 1.8, 6500 1.8, 60 2.0 and 90 2.0 series joists. Additional support may be required for other loadings. See BC CALC<sup>®</sup> software.
- Contact Boise Cascade EWP Engineering for reinforcement requirements on BCI<sup>®</sup> Joist depths greater than 16".



# Brick Ledge Load Bearing Cantilever Details



# Non-Load Bearing Wall Cantilever Details

BCI<sup>®</sup> Joists are intended only for applications that provide permanent protection from the weather.



- These details apply to cantilevers with uniform loads only.
- It may be possible to exceed the limitations of these details by analyzing a specific application with the BC CALC<sup>®</sup> software.

Fasten the 2x8 minimum to the BCl<sup>®</sup> Joist by nailing through the backer block and joist web with 2 rows of 3" (10d) nails at 6" on center. Use  $3\frac{1}{2}$ " (16d) nails with BCl<sup>®</sup> 90 2.0 joists. Clinch all nails.



# **Reinforced Load Bearing Cantilever Table**

0 No Reinforcement Required WS Web Stiffeners at Support

## **BCI®** Joists

Web Stiffeners Plus One Reinforcer Web Stiffeners Plus Two Reinforcers

Use Deeper Joists or Closer Spacing

2

х

		Roof			ç	Specified	Snow L	.oad [pst	f]		
Joist		Truss		30		i i	40		ĺ	50	
Depth	Joist	Span				Joist	t Spacing	g (in)			
[in]	Series	[ft]	16	19.2	24	16	19.2	24	16	19.2	24
		24	0	0	WS	0	WS	Х	0	Х	Х
		26	0	0	Х	0	Х	Х	0	Х	Х
		28	0	0	X	0	X	X	X	X	X
	BCI®	30	0	0	X	0	X	X	X	X	X
	5000	32	0	 	X	X	X	X	X	X	X
	1.7	36	0	X	X	X	X	X	X	X	X
		38	0	X	X	X	X	X	X	X	X
		40	0	X	X	X	X	X	X	X	X
		42	0	Х	Х	Х	Х	Х	Х	Х	Х
		24	0	0	0	0	0	WS	0	0	X
		26	0	0	0	0	0	1	0	WS	X
		28	0	0	WS	0	0	X	0	ws	X
=	BCI®	32	0	0	WS	0	0	X	0	×	X
91/2	6000	34	0	0	1	0	WS	X	0	X	X
	1.8	36	0	0	X	0	1	X	ŴS	X	X
		38	0	0	Х	0	Х	Х	Х	Х	Х
		40	0	0	Х	0	Х	Х	Х	Х	Х
		42	0	WS	Х	0	Х	Х	Х	Х	X
		24	0	0	0	0	0	WS	0	0	1
		26	0	0	0	0	0	1	0	WS	X
	DOIR	20	0	0	WS WS	0	0	1	0	1	X
	BCI®	32	0	0	WS	0	WS	X	0	1	X
	6500	34	0	0	1	0	WS	X	0	X	X
	1.8	36	0	0	1	0	1	X	WS	X	X
		38	0	0	1	0	1	Х	1	Х	Х
		40	0	0	2	0	1	Х	1	Х	Х
		42	0	WS	Х	0	Х	Х	Х	Х	Х
		24	0	0	WS	0	0	WS	0	WS	X
		26	0	0	WS	0	WS	X	0	WS	X
		28	0	0	WS	0	WS	X	0	ws v	X
	BCI®	32	0	0	WS	0	WS	X	WS	×	^ X
	5000	34	0	WS	WS	0	WS	X	WS	X	X
	1.7	36	0	WS	X	ŴS	X	X	WS	X	X
		38	0	WS	Х	WS	Х	Х	Х	Х	Х
		40	0	WS	Х	WS	Х	Х	Х	Х	Х
		42	0	WS	Х	WS	Х	Х	Х	Х	Х
		24	0	0	0	0	0	WS	0	0	WS
		26	0	0	0	0	0	WS	0	0	1
		28	0	0	WS	0	0	WS	0	WS	1
	BCI®	32	0	0	WS	0	0	1	0	WS	X
	6000	34	0	0	WS	0	WS	1	0	1	X
	1.8	36	0	0	WS	0	WS	1	0	1	X
		38	0	0	WS	0	WS	Х	WS	1	Х
		40	0	0	1	0	WS	Х	WS	Х	Х
		42	0	0	1	0	1	Х	1	Х	Х
		24	0	0	0	0	0	WS	0	0	WS
		26	0	0	0	0	0	WS	0	WS	1
		28	0	0	WS	0	0	WS	0	WS	1 
	BCI®	30	0	0	WS	0	0	1	0	WS	X
17	6500	34	0	0	WS	0	WS	1	0	1	X
-	1.8	36	0	0	WS	0	WS	1	WS	1	X
		38	0	0	WS	0	WS	Х	WS	1	Х
		40	0	0	1	0	WS	Х	WS	2	Х
		42	0	WS	1	0	1	Х	1	2	Х
		24	0	0	0	0	0	WS	0	0	WS
		26	0	0	WS	0	0	WS	0	WS	1
	DOIR	20	0	0	WS	0	0	WS WS	0	WS	Y
	BCI®	32	0	0	WS	0	WS	1	0	WS	X
	60	34	0	0	WS	0	WS	1	0	1	X
	2.0	36	0	0	WS	0	WS	X	WS	1	X
		38	0	0	WS	0	WS	Х	WS	1	Х
		40	0	0	1	0	WS	Х	WS	2	X
		42	0	WS	1	0	1	X	1	2	X
		24	0	0	0	0	0	0	0	0	WS
		20	0	0	0	0	0	0	0	0	1
	DOI®	30	0	0	0	0	0	WS	0	0	2
	BCI <sub>2</sub>	32	0	0	0	0	0	1	0	0	X
	90	34	0	0	0	0	0	1	0	0	X
	2.0	36	0	0	0	0	0	2	0	1	X
		38	0	0	WS	0	0	2	0	2	Х
		40	0	0	WS	0	0	Х	0	2	X
		42	0	0	1	0	0	Х	0	Х	X
4	1 × 1						ston the r		A		

specified floor dead load, 40 psf specified floor
live load, 100 plf specified wall dead load, 10 psf
snow load (Standard Term Load Duration).

2. Cut 48" long reinforcers to match the joist depth. List out  $2^{-1}/2^{-1}$  plywood/OSB-rated sheathing, Exposure 1, 48/24 Span Rating panels. The face grain must be horizontal (measure the  $48^{\circ}$  dimension along the long edge of the panel).

Stiffener Nailing Schedule on page 9. 5. Use the BC CALC<sup>®</sup> software to analyze conditions that are not covered by this table. It may be possible to exceed the limitations of this table by analyzing a specific application with BC CALC<sup>®</sup> software.

40

42

0

WS

WS

0 0

0

WS

stagger the nails to avoid splitting the joist flanges

Attach web stiffeners per intermediate Web

Specified Snow Load [psf] Roof 30 40 50 Joist Truss Joist Series Joist Spacing [in] Depth Span [in] [ft] 16 19.2 24 16 19.2 24 19.2 24 16 WS WS 24 WS WS 0 0 0 0 0 1 26 28 WS WS 0 X X X X X X ŴŠ ŴS ŴŠ WS 30 32 WS WS WS WS WS WS **BCI**<sup>®</sup> ws ws C X X 5000 WS WS 34 C 1.7 WS WS WS X X WS WS 36 0 XX ŴS WS WS 38 0 WS WS 40 WS ŴŠ ŴŠ 42 ŴS WS 24 0 0 0 26 28 WS WS 0 0 0 WS 0 ws ws 0 30 WS WS WS **BCI**<sup>®</sup> 32 0 C WS 0 0 WS WS 6000 34 WS WS WS ws 1.8 36 38 WS WS WS X X X X WS WS WS WS WS WS WS WS WS 40 42 24 0 WS WS 0 0 WS WS 26 0 WS 0 0 WS WS 28 0 WS 0 30 WS WS BCI® 0 32 WS WS WS 4 X X X X 6500 34 0 0 WS WS WS 0 WS WS WS WS 1.8 WS 36 WS 0 38 WS WS WS WS 40 WS WS X WS WS WS 42 WS WS 24 0 0 0 0 26 28 ŴS WS ŴS WS 0 0 WS WS WS WS 0 0 WS ٢ BCI® 30 32 WS 0 WS n 0 WS WS WS 60 34 WS WS WS WS ٢ 2.0 36 38 WS WS WS WS WS WS WS WS WS X X X X X n 0 WS 40 WŚ 42 WS WS WS ŵs 24 0 0 0 0 0 WS 26 28 0 0 0 0 ŴS ŴŠ 0 0 0 0 WS WS BCI® 30 0 0 WS 32 0 0 1 0 0 90 34 ŴŠ ŴS 0 0 2.0 36 WS WS WS 0 2 X X X X 38 40 WS WS Λ WS 0 0 n WS WS WS WS 0 0 4 WS 24 0 0 WS 0 WS WS WS 26 28 0 0 0 WS n WS WS 0 0 WS 30 WS 0 WS WS BCI® 32 34 WS WS Ω WS 1 ٢ n 6000 WS WS WS WS ٢ 1 1.8 36 WS WS WS WS X X X X 38 ŴŠ WS ŴS WS WS WS WS WS 40 WS WS 42 WS 1 24 0 0 WS ŴS 26 0 WS WS 0 0 WS WS WS 28 0 0 WS 0 WS WS WS 30 WS BCI® 0 32 WS WS W 1 X X X X X WS 6500 WS 34 Λ WS 0 WS WS WS WS WS 1.8 0 36 38 WS WS WS WS WS 0 n ŴS 0 40 WS WS WS WS 42 24 WS WS WS WS 16" ŵs 0 n 0 0 0 ŴŚ WS WS 26 28 0 0 WS WS 0 С WS WS 30 32 WS WS WS 0 0 WS BCI® WS WS 1 X 60 34 WS WS WS WS 2.0 WS WS WS WS WS 36 WS WS X X X X 38 0 WS WS ws 40 WS WS 42 WS WS Ŵ WS ŴS 24 0 0 0 0 0 0 26 ŴS WS 0 0 0 WS WS 28 0 0 0 WS WS 30 0 BCI WS WS 32 0 0 0 0 90 WS WS WS WS 34 0 0 2.0 36 WS 0 WS 0 WS WS 0 WS WS WS 38 Λ 0 WS 0 0 0 WS

WS

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34'

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# **Roof Framing**

# **BCI®** Rafters



#### SAFETY WARNING

DO NOT ALLOW WORKERS ON BCI® JOISTS UNTIL ALL HANGERS, BCI® RIM JOISTS, RIM BOARDS, BCI® BLOCKING PANELS, X-BRACING AND TEMPORARY 1x4 STRUT LINES ARE INSTALLED AS SPECIFIED BELOW.

- · Build a braced end wall at the end of the bay, or permanently install the first eight feet of BCI® Joists and the first course of sheathing. As an alternate, temporary sheathing may be nailed to the first four feet of BCI® Joists at the end of the bay.
- · All hangers, BCI® rim joists, rim boards, BCI® blocking panels, and x-bracing must be completely installed and properly nailed as each BCI<sup>®</sup> Joist is set.
- · Install temporary 1x4 strut lines at no more than eight feet on center as additional BCI® Joists are set. Nail the strut lines to the sheathed area, or braced end wall, and to each BCI® Joist with 2-21/2" (8d) nails.

BCI® Ceiling Joist with Bevel End Cut (For Limited-Access Attics Only)

SERIOUS ACCIDENTS CAN RESULT FROM INSUFFICIENT ATTENTION TO PROPER BRACING DURING CONSTRUCTION. ACCIDENTS CAN BE AVOIDED UNDER NORMAL CONDITIONS BY FOLLOWING THESE GUIDELINES:

- · The ends of cantilevers must be temporarily secured by strut lines on both the top and bottom flanges.
- Straighten the BCI<sup>®</sup> Joists to within <sup>1</sup>/<sub>2</sub> inch of true alignment before attaching strut lines and sheathing.
- Remove the temporary strut lines only as required to install the permanent sheathing.
- Failure to install temporary bracing may result in sideways buckling or roll-over under light construction loads.





#### NOTES:

- 1) Detail is to be used only for ceiling joists with no access to attic space.
- 2) Ceiling joist must be designed to carry all roof load transferred through rafter struts as shown.
- 3) BCI® ceiling joist end reaction may not exceed 550 pounds.
- 4) Minimum roof slope is 6/12.
- 5) Nail roof rafter to BCI® top flange with 1-31/2" (16d) sinker or box nail.
- 6) 1x4 nailers must be continuous and nailed to a braced end wall.
- 7) Install a web stiffener on each side of BCI<sup>®</sup> Joist at beveled ends. Nail roof rafter to BCI<sup>®</sup> Joist per building code requirements for ceiling joist to roof rafter connection.

# **Roof Framing Details**

### Additional roof framing details available with BC FRAMER® software



#### LATERAL SUPPORT

BCI® Joists must be laterally supported at end supports (including supports adjacent to overhangs) with hangers, rimboard, or blocking (VERSA-LAM<sup>®</sup>, Boise Cascade Rimboard or BCI<sup>®</sup> Joist). Metal cross bracing or other x-bracing provides adequate lateral support for BCI<sup>®</sup> Joists, consult governing building code for roof diaphragm connection provisions.

#### **MINIMUM BEARING LENGTH FOR BCI®** JOISTS

- Minimum bearing length at end support is  $1^{1/2}$  for BCI® 5000, 6000 and 6500, and  $1^{3/4}$  for BCI® 60 and 90.  $3\frac{1}{2}$  inches is required at cantilever and intermediate supports.
- Longer bearing lengths allow higher reaction values. Refer to the building code evaluation report or the BC CALC<sup>®</sup> software.

#### NAILING REQUIREMENTS

- BCI® rim joist, rim board or closure panel to BCI® joist:
- Rims or closure panel  $1\frac{3}{4}$  inches thick and less:  $2-2\frac{1}{2}$ " (8d) nails, one each in the top and bottom
- 2-2½" (8d) halls, one each in the top and bottom flange. BCI® 5000 rim joist: 2-3" (10d) box nails, one each in the top and bottom flange. BCI® 6000, 60 rim joist: 2-3½" (16d) box nails, one each in the top and bottom flange. BCI® 6500, 90 rim joist: Toe-nail top flange to rim joist with 2-3" (10d) box nails, one each side of flange.
- BCI® rim joist, rim board or BCI® blocking panel to support:
- 21/2" (8d) nails at 6 inches on center.
- When used for shear transfer, follow the building designer's specification.

- BCI® joist to support:
  - $2\text{-}2^{1}$  (8d) nails, one on each side of the web, placed  $1^{1}\!\!/_2$  inches minimum from the end of the BCI® Joist to limit splitting.
- Sheathing to BCI® joist:
  - BCI® 5000 joist: Maximum nail spacing is 18 inches on center.
  - BCI® 6000, 6500, 60, 90 joist: Maximum nail spacing is 24 inches on center. 14 gauge staples may be substituted for 2½" (8d)
- nails if the staples penetrate at least 1 inch into the joist. Wood screws may be acceptable, contact local
- building official and/or Boise Cascade EWF Engineering for further information.

#### BACKER AND FILLER BLOCK DIMENSIONS

Series	Backer Block Thickness	Filler Block Thickness
5000 1.7	<sup>3</sup> ⁄4" or <sup>7</sup> ∕8" wood panels	Two ¾" wood panels or 2 x _
6000 1.8	1 <sup>1</sup> / <sub>8</sub> " or two <sup>1</sup> / <sub>2</sub> " wood panels	2 x _ + 5⁄8" or 3⁄4" wood panel
6500 1.8	1 <sup>1</sup> / <sub>8</sub> " or two <sup>1</sup> / <sub>2</sub> " wood panels	2 x _ + 5⁄8" or 3⁄4" wood panel
60 2.0	1 <sup>1</sup> / <sub>8</sub> " or two <sup>1</sup> / <sub>2</sub> " wood panels	2 x _ + 5⁄8" or 3⁄4" wood panel
90 2.0	2 x _ lumber	Double 2 x _ lumber

Cut backer and filler blocks to a maximum depth equal to the web depth minus 1/4" to avoid a forced

#### WEB STIFFENER REQUIREMENTS

See Web Stiffener Requirements on page 9.

#### MAXIMUM SLOPE

Unless otherwise noted, all roof details are valid for slopes of 12 in 12 or less.

#### VENTILATION

The 1½ inch, pre-stamped knock-out holes spaced at 12 inches on center along the BCI<sup>®</sup> Joist may all be knocked out and used for cross ventilation. Deeper joists than what is structurally needed may be advantageous in ventilation design. Consult local building official and/or ventilation specialist for specifc ventilation requirements.

#### **BIRDSMOUTH CUTS**

BCI<sup>®</sup> Joists may be birdsmouth cut only at the low end support. BCI<sup>®</sup> joists with birdsmouth cuts may cantilever up to 2'-6" past the low end support. The bottom flange must sit fully on the support and may not overhang the inside face of the support. High end supports and intermediate supports may not be birdsmouth cut. birdsmouth cut.

#### **PROTECT BCI® JOISTS FROM THE** WEATHER

BCI® Joists are intended only for applications that Provide permanent protection from the weather. Bundles of BCI® Joists should be covered and stored off of the ground on stickers.

Western Specifier Guide - CANADA

# **Roof Span Tables**

Loado	Is Series De		Lov	w Roof Slo	ре	High Roof Slope			
LUaus	Series	(in)	16"	19.2"	24"	16"	19.2"	24"	
		9½"	19'-1"	18'-2"	17'-2"	16'-10"	16'-1"	15'-3"	ŀ
	5000 1.7	111⁄8"	21'-9"	20'-9"	19'-7"	19'-2"	18'-4"	17'-4"	].
		14"	23'-10"	22'-9"	21'-6"	21'-0"	20'-1"	19'-0"	
		9½"	19'-11"	19'-0"	17'-11"	17'-7"	16'-10"	15'-10"	1
psf psf	6000 4 9	117⁄8"	22'-8"	21'-7"	20'-5"	20'-0"	19'-1"	18'-1"	
10 20	0000 1.0	14"	24'-11"	23'-9"	22'-6"	22'-0"	21'-0"	19'-10"	ľ
		16"	26'-10"	25'-8"	24'-2"	23'-9"	22'-8"	21'-5"	
.oac		9½"	25'-6"	24'-4"	23'-0"	22'-6"	21'-6"	20'-4"	
ad L wo	6500 1 9	117⁄8"	27'-5"	26'-2"	24'-9"	24'-3"	23'-2"	21'-11"	
Dea	0000 1.0	14"	29'-4"	28'-0"	26'-5"	25'-11"	24'-9"	23'-4"	
ied		16"	31'-1"	29'-8"	28'-1"	27'-6"	26'-3"	24'-10"	
ecifi		111⁄8"	28'-11"	27'-7"	26'-1"	25'-7"	24'-5"	23'-1"	ŀ
Sp. Sp.	60 2.0	14"	30'-11"	29'-6"	27'-10"	27'-4"	26'-1"	24'-8"	
		16"	32'-10"	31'-4"	29'-7"	29'-0"	27'-8"	26'-2"	
		111/8"	31'-10"	30'-4"	28'-8"	28'-2"	26'-10"	25'-4"	
	90 2.0	14"	34'-0"	32'-5"	30'-7"	30'-1"	28'-8"	27'-1"	1
		16"	36'-0"	34'-5"	32'-6"	31'-10"	30'-5"	28'-9"	ŀ
		9½"	17'-9"	16'-11"	16'-0"	15'-10"	15'-1"	14'-3"	
	5000 1.7	117⁄8"	20'-3"	19'-4"	18'-3"	18'-0"	17'-2"	16'-3"	ŀ
		14"	22'-2"	21'-2"	20'-0"	19'-8"	18'-10"	17'-9"	
		9½"	18'-6"	17'-8"	16'-8"	16'-6"	15'-9"	14'-10"	
psf psf	6000 4 9	111/8"	21'-1"	20'-1"	19'-0"	18'-9"	17'-11"	16'-11"	ľ
10   30	6000 1.8	14"	23'-2"	22'-2"	20'-11"	20'-7"	19'-8"	18'-7"	ŀ
		16"	25'-0"	23'-10"	22'-6"	22'-3"	21'-3"	20'-0"	1
oac		9½"	23'-9"	22'-8"	21'-5"	21'-1"	20'-2"	19'-0"	
	0500 4 0	111/8"	25'-7"	24'-5"	23'-0"	22'-9"	21'-8"	20'-6"	
Dea	6500 1.8	14"	27'-4"	26'-1"	24'-7"	24'-3"	23'-2"	21'-10"	
ed		16"	29'-0"	27'-8"	26'-1"	25'-9"	24'-7"	23'-3"	ŀ
ecifi		111/8"	26'-11"	25'-8"	24'-3"	23'-11"	22'-10"	21'-7"	
Spe Spe	60 2.0	14"	28'-9"	27'-6"	25'-11"	25'-7"	24'-5"	23'-1"	1
		16"	30'-7"	29'-2"	27'-6"	27'-2"	25'-11"	24'-6"	1
		111/8"	29'-7"	28'-3"	26'-8"	26'-4"	25'-2"	23'-9"	1
	90 2.0	14"	31'-8"	30'-2"	28'-6"	28'-2"	26'-10"	25'-4"	1
		16"	33'-6"	32'-0"	30'-2"	29'-10"	28'-6"	26'-10"	

Spans apply to simple span application only.

For BCI<sup>®</sup> 5000, 6000 and 6500, the minimum end bearing lengths are  $1\frac{1}{2}$ " at high end and  $3\frac{1}{2}$ " at lower end.

For BCI<sup>®</sup> 60 and 90, the minimum end bearing lengths are  $1\frac{3}{4}$ " at high end and  $3\frac{1}{2}$ " at lower end.

Maximum spans are measured, centerline to centerline of bearing, on horizontal projection, and based on uniformly loaded joists.

Live load deflection is limited to L/240. Total load deflection is limited to L/180 or 1".

Refer to appropriate sections of the BCI® Specifier Guide for installation guidelines and construction details.

Allowable spans assume no composite action provided by sheathing.

• Low roof slope is from 1/4/12 to 6/12.

• High roof slope is from 6/12 to 12/12.

Table assumes a 2 foot roof overhang.

It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC<sup>®</sup> software.

• Slope roof joists at least ¼" over 12" to minimize ponding.

Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

## WARNING: Use of Span Tables for Commercial Projects (NBCC2005: Part 4)

All projects within the scope of Part 4 of the National Building Code of Canada (NBCC) must consider the effects of concentrated loads, as stipulated in article 4.1.5.10. The designer of record must verify the effects of a concentrated load on the joists on all projects within the scope of Part 4 of NBCC (2005). Table 4.1.5.10 in NBCC (2005) lists concentrated loads that shall be analyzed with respect to the intended use of the floor. Given the numerous possible permutations, the span tables listed above do not take the effects of concentrated loads into consideration.

# **Roof Span Tables**

Loodo	Sorioo	Depth	Lov	w Roof Slo	ре	Hig	h Roof Slo	ре
LUAUS	Selles	(in)	16"	19.2"	24"	16"	19.2"	24"
		9½"	16'-9"	16'-0"	14'-9"	15'-0"	14'-4"	13'-6"
	5000 1.7	111⁄8"	19'-1"	18'-3"	16'-9"	17'-1"	16'-3"	15'-5"
		14"	21'-0"	20'-0"	18'-3"	18'-8"	17'-10"	16'-10"
		9½"	17'-6"	16'-9"	15'-9"	15'-8"	14'-11"	14'-1"
psf psf	0000 4 0	111/8"	19'-11"	19'-0"	18'-0"	17'-9"	17'-0"	16'-1"
10	0000 1.8	14"	21'-11"	20'-11"	19'-9"	19'-7"	18'-8"	17'-8"
		16"	23'-8"	22'-7"	21'-4"	21'-1"	20'-2"	19'-0"
oac		9½"	22'-5"	21'-5"	20'-2"	20'-0"	19'-1"	18'-1"
l bi	05004.0	111/8"	24'-2"	23'-1"	21'-9"	21'-7"	20'-7"	19'-5"
Dea Snc	6500 1.8	14"	25'-10"	24'-8"	23'-3"	23'-0"	22'-0"	20'-9"
ed		16"	27'-5"	26'-2"	24'-8"	24'-5"	23'-4"	22'-0"
ecifi		111⁄8"	25'-5"	24'-3"	22'-11"	22'-8"	21'-8"	20'-6"
Spe Spe	60 2.0	14"	27'-3"	25'-11"	24'-6"	24'-3"	23'-2"	21'-11"
		16"	28'-11"	27'-7"	26'-0"	25'-9"	24'-7"	23'-3"
		111⁄8"	28'-0"	26'-8"	25'-2"	25'-0"	23'-10"	22'-6"
	90 2.0	14"	29'-11"	28'-6"	26'-11"	26'-8"	25'-6"	24'-0"
		16"	31'-8"	30'-3"	28'-6"	28'-4"	27'-0"	25'-6"
		9½"	16'-0"	15'-0"	13'-5"	14'-4"	13'-8"	12'-11"
	5000 1.7	111⁄8"	18'-3"	17'-1"	15'-3"	16'-4"	15'-7"	14'-9"
		14"	20'-0"	18'-7"	16'-7"	17'-11"	17'-1"	16'-2"
		9½"	16'-9"	15'-11"	15'-1"	15'-0"	14'-3"	13'-6"
psf psf	6000 1 9	111⁄8"	19'-0"	18'-2"	17'-2"	17'-0"	16'-3"	15'-4"
10 50	0000 1.0	14"	20'-11"	20'-0"	18'-10"	18'-9"	17'-11"	16'-11"
ш Ш С С		16"	22'-7"	21'-6"	20'-4"	20'-2"	19'-3"	18'-2"
-oa(		9½"	21'-5"	20'-5"	19'-3"	19'-2"	18'-3"	17'-3"
ad L ow l	6500 1 8	111⁄8"	23'-1"	22'-0"	20'-9"	20'-8"	19'-8"	18'-7"
Dea	0000 1.0	14"	24'-8"	23'-6"	22'-2"	22'-1"	21'-0"	19'-10"
ied		16"	26'-2"	25'-0"	23'-7"	23'-5"	22'-4"	21'-1"
ecif ecif		117⁄8"	24'-4"	23'-2"	21'-10"	21'-9"	20'-9"	19'-7"
Spi	60 2.0	14"	26'-0"	24'-9"	23'-5"	23'-3"	22'-2"	20'-11"
		16"	27'-7"	26'-4"	24'-10"	24'-8"	23'-6"	22'-3"
		117⁄8"	26'-9"	25'-6"	23'-2"	23'-11"	22'-10"	21'-7"
	90 2.0	14"	28'-6"	27'-3"	25'-8"	25'-7"	24'-4"	23'-0"
		16"	30'-3"	28'-10"	27'-3"	27'-1"	25'-10"	24'-5"

• Spans apply to simple span application only.

• For BCI<sup>®</sup> 5000, 6000 and 6500, the minimum end bearing lengths are 1<sup>1</sup>/<sub>2</sub>" at high end and 3<sup>1</sup>/<sub>2</sub>" at lower end.

 For BCI<sup>®</sup> 60 and 90, the minimum end bearing lengths are 1<sup>3</sup>⁄<sub>4</sub>" at high end and 3<sup>1</sup>⁄<sub>2</sub>" at lower end.

 Maximum spans are measured, centerline to centerline of bearing, on horizontal projection, and based on uniformly loaded joists.

• Live load deflection is limited to L/240. Total load deflection is limited to L/180 or 1".

 Refer to appropriate sections of the BCI<sup>®</sup> Specifier Guide for installation guidelines and construction details.

 Allowable spans assume no composite action provided by sheathing.

• Low roof slope is from 1/4/12 to 6/12.

• High roof slope is from 6/12 to 12/12.

 Table assumes a 2 foot roof overhang.

 It may be possible to exceed the limitations of this table by analyzing a specific application with the BC CALC<sup>®</sup> software.

• Slope roof joists at least ¼" over 12" to minimize ponding.

 Allowable spans and loads shall be adjusted and checked for wind load as required by local building code.

## WARNING: Use of Span Tables for Commercial Projects (NBCC2005: Part 4)

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# **Limit States Design - Standard Term Load Duration** Deflection Criteria: L/240 (Live Load) & L/180 (Total Load)

		BCI <sup>®</sup> 5000 1.7 Series													
				2" F	lange W	idth									
	9½'	' BCI® 5000	1.7	111/3	" BCI® 5000	) 1.7	14"	BCI® 5000	1.7						
	Deflection (	unfactored)		Deflection (	unfactored)		Deflection (	unfactored)							
Span Length [ft]	L/240 (Live)	L/180 (Total)	Factored Strength Resistance	L/240 (Live)	L/180 (Total)	Factored Strength Resistance	L/240 (Live)	L/180 (Total)	Factored Strength Resistance						
8			336			357			376						
10			268			285			300						
12			224			285			300						
14			168			204			214						
16	81		129			165			188						
18	58		101			130			155						
20	42		82			106			125						
22	32	43	68	53		87			103						
24	25	33	57	41		73			87						
26	19	26	48	33		62	47		74						

	PSF to PLF CONVERSION TABLE												
Joist		LOAD (psf)											
Spacing	20	20 25 30 35 40 45 50 60											
12"	20	25	30	35	40	45	50	60					
16"	27	33	40	47	53	60	67	80					
19.2"	32	40	48	56	64	72	80	96					
24"	40	50	60	70	80	90	100	120					

TO CONVERT FROM SPECIFIED LOAD (PLF) TO FACTORED LOAD (PLF) - Factored (PLF) = 1.25 x Specified Dead Load (PLF) + 1.50 x Specified Live/Snow Load (PLF)

### **General Notes**

- 1. The spans listed are the clear span distance between supports.
- 2. Table is valid for simple and continuous span applications under uniform load.
- 3. Design of continuous spans shall be based on the longest span. The shorter span shall not be less than 50% of the longest length.
- 4. Table assumes a fully laterally restrained compression flange.
- 5. The repetitive member factor,  ${\rm K}_{\rm H}$  is not applicable.
- 6. Table assumes a minimum bearing length with no stiffeners.

DEAD LOAD SLOPE FACTOR												
Joist Pitch	2/12	3/12	4/12	5/12	6/12	7/12	8/12	9/12	10/12	11/12	12/12	
Slope Factor	1.014	1.031	1.054	1.083	1.118	1.158	1.202	1.250	1.302	1.357	1.414	

# How to calculate a roof plf loading:

Joist: BCI® 5000, 9½" Spacing: 24" o.c. Span: 16'-0" Roof Pitch: 4/12

Snow Load : SL = 30 lbs/ft<sup>2</sup>

Dead Load : DL = 10 lbs/ft<sup>2</sup>

#### **Design Criteria:**

Live Load Deflection: L/240 Total Load Deflection: L/180 Applied Load: WSL = SL x tributary width

WSL =  $\frac{30 \text{ lbs}}{\text{ft}^2}$  x  $\frac{24 \text{ in}}{12 \text{ in / ft}}$ WSL = 60 plf (lbs/ft)

WDL = DL x tributary width x slope factor

 $WDL = \frac{10 \text{ lbs}}{\text{ft}^2} \times \frac{24 \text{ in}}{12 \text{ in} / \text{ft}} \times 1.054$ WDL = 21.1 plf (lbs/ft)

WSL = WSL = 60 plf WTL = WSL + WDL = 81.1 plf WF = 1.25 x WDL + 1.50 x WSL = 117 plf (Factored)

### Check Capacities :

Live Load (L/240): 60 plf < 81 plf -> ok Total Load (L/180): 81.1 plf < -- -> ok Factored Resistance: 117 plf <129 plf -> ok

#### Note:

For roof pitches greater than a 2/12, approximate the increased dead load by multiplying the specified dead load by the slope factor.

#### **Definitions:**

WSL	Uniform Snow Load	[lb/ft]
WDL	Uniform Dead Load	[lb/ft]
WTL	Uniform Total Load	[lb/ft]
WF	Uniform Factored Load	[lb/ft]

# Limit States Design - Standard Term Load Duration Deflection Criteria: L/240 (Live Load) & L/180 (Total Load)

				В	Cl <sup>®</sup> 60	000 1.8	8 Seri	es				
					<b>2</b> ⁵/	16 <b>" Flar</b>	nge Wie	dth				
	9½"	BCI® 600	0 1.8	111/8"	BCI® 600	00 1.8	14" E	3CI® 600	0 1.8	16" E	3CI® 600(	0 1.8
	Defle (unfac	ection ctored)	Strength ance	Defle (unfac	ection stored)	Strength ance	Defle (unfac	ection tored)	Strength ance	Defle (unfac	ection stored)	Strength ance
Span Length [ft]	L/240 (Live)	L/180 (Total)	Factored ( Resist	L/240 (Live)	L/180 (Total)	Factored ( Resist	L/240 (Live)	L/180 (Total)	Factored ( Resist	L/240 (Live)	L/180 (Total)	Factored ( Resist
8			389			402			414			425
10			311			321			331			340
12			311			321			331			340
14	139		216			229			236			242
16	95		165			201			207			212
18	68		131	112		168			184			188
20	50	67	106	83		136			161			170
22	38	50	87	63		112			133			152
24	29	39	73	49		94	71		112			128
26	23	31	62	38	51	80	56		95			109

	BCI <sup>®</sup> 6500 1.8 Series													
					2º/	16 <b>" Flar</b>	nge Wio	dth						
	9½"	3CI® 650	0 1.8	111/8"	BCI® 650	00 1.8	14" E	3CI® 650	0 1.8	16" E	3CI® 6500	0 1.8		
	Defle (unfac	ction tored)	Strength ance	Defle (unfac	ection stored)	Strength ance	Deflection (unfactored)		Strength ance	Defle (unfac	ction tored)	Strength ance		
Span Length [ft]	L/240 (Live)	L/180 (Total)	Factored	L/240 (Live)	L/180 (Total)	Factored	L/240 (Live)	L/180 (Total)	Factored	L/240 (Live)	L/180 (Total)	Factored Resist		
8			389			402			414			425		
10			311			321			331			340		
12			259			268			276			283		
14			222			229			236			242		
16	105		183			201			207			212		
18	75		145			178			184			188		
20	55	73	117	91		151			165			170		
22	41	55	97	69		124			147			154		
24	32	43	81	54		104	77		124			141		
26	25	26	69	43	57	89	61		105			120		

# Limit States Design - Standard Term Load Duration Deflection Criteria: L/240 (Live Load) & L/180 (Total Load)

			E	<b>BCI<sup>®</sup> 60</b>	2.0 Seri	es			
				<b>2</b> <sup>5</sup> / <sub>16</sub> "	Flange V	Vidth			
	113	‰" BCI® 60	2.0	14	4" BCI® 60 2	2.0	16	60 2® 80 8	2.0
	Deflection (	unfactored)	trength nce	Deflection (	unfactored)	trength nce	Deflection (	trength nce	
Span Length [ft]	L/240 (Live)	L/180 (Total)	Factored S Resista	L/240 (Live)	L/180 (Total)	Factored S Resista	L/240 (Live)	L/180 (Total)	Factored S Resista
8			402			414			425
10			321			331			340
12			268			276			283
14			229			236			242
16			201			207			212
18			178			184			188
20			160			165			170
22	83		146			150			154
24	65	86	134			138			141
26	51	68	122	75		127			130

	BCI <sup>®</sup> 90 2.0 Series															
		3 <sup>1</sup> / <sub>2</sub> " Flange Width														
	111/8"	BCI <sup>®</sup> 9	0 2.0	14" I	BCI® 90	) 2.0	16"	BCI® 90	) 2.0	18"	BCI® 90	2.0	20"	BCI® 90	) 2.0	
	Defle (unfac	ection tored)	Strength ance	Defle (unfac	ection tored)	Strength ance	Deflection (unfactored)		Defle (unfac	ection tored)	Strength ance	Defle (unfac	ection ctored)	Strength ance		
Span Length [ft]	L/240 (Live)	L/180 (Total)	Factored Resist	L/240 (Live)	L/180 (Total)	Factored Resist	L/240 (Live)	L/180 (Total)	Factored Resist	L/240 (Live)	L/180 (Total)	Factored Resist	L/240 (Live)	L/180 (Total)	Factored Resist	
8			485			487			490			722			764	
10			388			389			392			577			611	
12			323			324			326			481			509	
14			277			278			280			412			436	
16			242			243			245			361			382	
18			215			216			217			320			339	
20			194			194			196			288			305	
22			176			177			178			262			277	
24	95		161			162			163			240			254	
26	75		149			149			150			222			235	



Dim Board Tupo	Thickness	φH	φV [lb/ft]		φZ	φP
Kiili Board Type	[in]	[lb/ft]	d ≤ 16"	d > 16"	[lb]	[lb]
Paisa Casaada Dimboard	1"	235	5500	2750	495	5840
Boise Cascade Rimboard	11⁄8"	235	7340	5000	585	5840
Boise Cascade Rimboard Plus	11⁄8"	260	8090	5340	585	5840
Boise Cascade VERSA-STRAND 0.8E	1¼"	310	9460	5820	830	8990
Boise Cascade VERSA-LAM 1.4 1800	1 <sup>5</sup> ⁄ <sub>16</sub> "	SEE NOTE 7	10000	9090	585	7420

- 1.  $\phi$ H = Factored horizontal (shear) load transfer capacity is based on the minimum nailing attachment schedule specified in NBCC 2005 and APA document D340CA.
- 2.  $\phi V$  = Factored uniform bearing (vertical) load resistance. The uniform bearing load shall be simultaneously satisfied with the concentrated vertical load resistance, when applicable.
- 3.  $\varphi$ Z = Factored lateral resistance of a ½ inch (12.7 mm) diameter lag screw.
- 4.  $\phi P$  = Factored concentrated vertical load resistance based on 4½ inch (114 mm) bearing length. The concentrated vertical load shall be Western Specifier Guide CANADA

simultaneously satisfied with the uniform bearing load capacity, when applicable.

- All tabulated values are applicable to the standard-term load duration and permitted to be adjusted for other load durations in accordance with CSA O86.
- 6. See CCMC Evaluation Report No. 13143 for further product information on Boise Cascade VERSA-STRAND 0.8E.
- 7. Refer to 1<sup>1</sup>/<sub>2</sub> inch (38 mm) D. Fir lumber at table 9.5.2 of CSA O86.

# **Fire Rating Assemblies**



### 3 Insulation

31/2" thick, minimum 2.5 pcf, mineral wool insulation batts.



### Gypsum Wallboard

One layer of <sup>5</sup>/<sub>8</sub>" Type C gypsum wallboard installed perpendicular to channels with end joints staggered 48". Boards to be fastened to channels with minimum 11/8" Type S drywall screws located 12" on center. Gypsum wallboard joints shall be covered with tape and coated with gypsum joint compound.

## **REFERENCE:**

PFS BCI®, Assembly 4

#### **1 Hour Fire Rating**

Floor/Ceiling Assembly STC 55 with Resilient Channels and Insulation STC 48 with Resilient Channels and WITHOUT Insulation



(3) Insulation (optional) 31/2" fiberglass batt insulation.

## 6 Gypsum Wallboard

Two layers of 1/2" Type C or 5/8" Type X gypsum wallboard. Base layer installed perpendicular to joists or channels and fastened with 13/4" screws located at 12" on center. Face layer installed parallel to base layer with end and edge joints staggered 16" minimum and fastened with 21/4" screws located at 12" on center on intermediate joists, and 8" on center at end joints. Gypsum wallboard joints shall be covered with tape and coated with gypsum joint compound.

### **REFERENCES:**

PFS BCI<sup>®</sup>, Assembly 1 PFS BCI<sup>®</sup>, Assembly 2

### Subfloor

Minimum 5%" plywood or OSB sheathing fastened to joists in accordance with Code specifications. Construction adhesive is optional.

### Structural Members

BCI® Joists having a minimum depth of 91/2" and spaced at 24" o.c. maximum.

### 4 Insulation Supports

Nominal 2x3 strapping located 16" o.c. or equivalent method to retain insulation above joist flanges.

### 6 Resilient Channels

Minimum 25 gauge 1/2" offset RC-1 galvanized steel channels installed perpendicular to joists spaced at 16" o.c. maximum and fastened with 11/2" screws at each joist intersection.

# SINGLE I-JOISTS - Canadian/Factored Resistance (Ibs)



			Top Mount Hange	' <b>s</b> <sup>5</sup>				F	ace Mount Hangers				
Joist	USP	Fast	tener Schedule <sup>4</sup>	SPF Uplift <sup>3</sup>	Down	100% <sup>2</sup>	USP	Faste	ener Schedule <sup>4</sup>	SPF Uplift <sup>3</sup>	Down	100% <sup>2</sup>	
Height	Stock No.1	Header	Joist	115%	DF	SPF	Stock No.1	Header	Joist	115%	DF	SPF	
BCI®	5000				Joi	st Width	n = 2″					-	
							THF20925 Min	(8) 10d	(2) 10d x 1-1/2	373	3050	2166	
9-1/2	TFL2095	(6) 10d	(2) 10d x 1-1/2	530	2495	1771	THF20925 Max	(12) 10d	(2) 10d x 1-1/2	373	3225	2290	
		<u> </u>					THE20112 Min	(8) 10d	(2) 10d x 1-1/2	373	3050	2166	
11-7/8	TFL20118	(6) 10d	(2) 10d x 1-1/2	530	2495	1771	THE20112 Max	(0) 10d	(2) 10d x 1-1/2	373	4135	2936	
							THE20140 Min	(12) 10d	(2) 10d x 1-1/2	373	3815	2709	
14	TFL2014	(6) 10d	(2) 10d x 1-1/2	530	2495	1771	THE20140 Max	(12) 10d	(2) 10d x 1-1/2	373	4045	2872	
BCI <sup>∞</sup>	60/6000				loist	 Width =	2-5/16"	(20) 100	(2) 100 X 1 112	010	1010	LOIL	
0.1/2	TEI 2305	(6) 10d	(2) 10d x 1-1/2	530	2/05	1771	THE23025	(12) 10d	(2) 10d x 1-1/2	238	3310	2350	
9-1/2	TEL 22110	(6) 100	(2) 10d x 1-1/2	520	2495	1771	THF23923	(12) 100	(2) 100 x 1-1/2	230 511	2210	2350	
11-7/0	TEL 22110	(6) 100	(2) 10d x 1-1/2	530	2495	1771	THF23110	(14) 100	(2) 100 x 1-1/2	511	3310	2300	
14	TEL 2214	(6) 100	(2) 10d x 1-1/2	530	2495	1771	THF23140	(10) 100	(2) 100 x 1-1/2	511	4405	3120	
16 RCI®	1FL2316	(6) 100	(2) 10d x 1-1/2	530	2495		1HF23160	(22) 100	(2) 100 x 1-1/2	511	4405	3128	
DCI 1/2	TEL 2505	(6) 10d	(2) 10d x 1 1/2	520	2405	- 1771	Z-9/10	(12) 104	(2) 10d x 1 1/2	220	2210	2250	
9-1/2	TFL2595	(0) 100	(2) 100 x 1-1/2	500	2495	4774	THF23923	(12) 100	(2) 100 X 1-1/2	230	0040	2350	
11-7/8	TFL25118	(6) 100	(2) 10d x 1-1/2	530	2495	1//1	THF25112	(14) 100	(2) 10d x 1-1/2	511	3310	2350	
14	TFL2514	(6) 10d	(2) 10d x 1-1/2	530	2495	1//1	THF25140	(18) 10d	(2) 10d x 1-1/2	511	4405	3128	
16 BCI®	IFL2516	(6) 10d	(2) 10d x 1-1/2	530	2495	1//1	THF25160	(22) 10d	(2) 10d x 1-1/2	511	4405	3128	
BOI	50	(10) 10 1		055	Joist		= 3-1/2	(10) 10 1			50.40	0700	
11-7/8	THO35118	(10) 10d	(2) 10d x 1-1/2	355	2975	2115	THF35112	(16) 10d	(2) 10d x 1-1/2	330	5240	3720	
14	THO35140	(12) 10d	(2) 10d x 1-1/2	355	4450	3160	THF35140	(20) 10d	(2) 10d x 1-1/2	330	6680	4743	
16	THO35160	(12) 10d	(2) 10d x 1-1/2	355	4450	3160	THF35157	(22) 10d	(2) 10d x 1-1/2	330	6680	4743	
18	TFI418	(6) 16d	(2) 10d x 1-1/2	385	4190	2975	THF35157	(22) 10d	(2) 10d x 1-1/2	330	6680	4743	
20	TFI420	(6) 16d	(2) 10d x 1-1/2	385	4190	2975	THF35157	(22) 10d	(2) 10d x 1-1/2	330	6680	4743	
			Skewed 45° Hang	ers				Sic	pe and Skew Hangers				
Joist	USP	Fast	tener Schedule*	SPF Uplift <sup>3</sup>	Down	100%	USP	Faste	ener Schedule⁴	SPF Uplift <sup>®</sup>	Down	100%	
Height	Stock No.'	Plate	Rafter	115%	DF	SPF	Stock No. ""	Header	Joist	115%	DF	SPF	
BCI®	5000	_			Joi	st Width	ו = 2″						
9-1/2	SKH2020L/R	(14) 10d	(10) 10d x 1-1/2	2065	2175	1545	LSSH20	(10) 10d	(7) 10d x 1-1/2	1030	2620	1860	
11-7/8	SKH2020L/R	(14) 10d	(10) 10d x 1-1/2	2065	2175	1545	LSSH20	(10) 10d	(7) 10d x 1-1/2	1030	2620	1860	
14	SKH2024L/R	(16) 10d	(10) 10d x 1-1/2	2065	4740	3365	LSSH20	(10) 10d	(7) 10d x 1-1/2	1030	2620	1860	
BCI <sup>®</sup> (	60/6000				Joist	Width =	2-5/16″						
9-1/2	SKH2320L/R	(14) 10d	(10) 10d x 1-1/2	2065	2175	1545	LSSH23	(10) 10d	(7) 10d x 1-1/2	1030	2620	1860	
11-7/8	SKH2320L/R	(14) 10d	(10) 10d x 1-1/2	2065	2175	1545	LSSH23	(10) 10d	(7) 10d x 1-1/2	1030	2620	1860	
14	SKH2324L/R	(16) 10d	(10) 10d x 1-1/2	2065	4740	3365	LSSH23	(10) 10d	(7) 10d x 1-1/2	1030	2620	1860	
16	SKH2324L/R	(16) 10d	(10) 10d x 1-1/2	2065	4740	3365	LSSH23 <sup>8</sup>	(10) 10d	(7) 10d x 1-1/2	1030	2620	1860	
BCI®	6500				Joist	Width =	2-9/16″						
9-1/2	SKH2520L/R	(14) 10d	(10) 10d x 1-1/2	2065	3265	2320	LSSH25	(14) 16d	(12) 10d x 1-1/2	1390	4260	3025	
11-7/8	SKH2520L/R	(14) 10d	(10) 10d x 1-1/2	2065	3265	2320	LSSH25	(14) 16d	(12) 10d x 1-1/2	1390	4260	3025	
14	SKH2524L/R	(16) 10d	(10) 10d x 1-1/2	2065	3265	2320	LSSH25	(14) 16d	(12) 10d x 1-1/2	1390	4260	3025	
16	SKH2524L/R	(16) 10d	(10) 10d x 1-1/2	2065	3265	2320	LSSH25 <sup>8</sup>	(14) 16d	(12) 10d x 1-1/2	1390	4260	3025	
BCI	90				Joist	Width =	= 3-1/2″	1					
11-7/8	SKH410L/R <sup>7</sup>	(16) 16d	(10) 16d	2530	3690	2620	LSSH35	(14) 16d	(12) 10d x 1-1/2	1845	5230	3715	
14	SKH414L/R7	(22) 16d	(10) 16d	2530	7405	5260	LSSH35	(14) 16d	(12) 10d x 1-1/2	1845	5230	3715	
16	SKH414L/R <sup>7</sup>	(22) 16d	(10) 16d	2530	7405	5260	LSSH35 <sup>8</sup>	(14) 16d	(12) 10d x 1-1/2	1845	5230	3715	
18	SKH414L/R7	(22) 16d	(10) 16d	2530	7405	5260	LSSH35 <sup>8</sup>	(14) 16d	(12) 10d x 1-1/2	1845	5230	3715	
20	SKH414L/R7	(22) 16d	(10) 16d	2530	7405	5260	LSSH35 <sup>8</sup>	(14) 16d	(12) 10d x 1-1/2	1845	5230	3715	
		ļ	Adjustable Height Hai	ngers									
Joist	USP	Fast	tener Schedule <sup>4</sup>	SPF Uplift <sup>3</sup>	Down	100% <sup>2</sup>	1) Shaded hangers requ	ire web stiffen	ers at ioist ends. Web st	iffeners may be			
Height	Stock No. 1,9	Header	Joist	115%	DF	SPF	required for non-shade	ed hangers by	I-ioist manufacturers				
BCI®	5000	noudor	loist Width = $2^{\circ}$			0.1	2) Factored resistance is	s hased on har	oper attachment to a DE	I or S-P-F spec	ios		
9-1/2 - 14							solid sawn boader S	ome loade ma	whe increased for durati	on of load adjust	monte		
BCI <sup>®</sup>	50/6000		loist Width = $2.5/2$	6″			Refer to USP Full Li	ne Catalon for	y de moreaseu for udrati · details	on or iodu dujuSi			
9-1/2							3) Uplift loads have been	n increased 15	% for wind and seismic	loading.			
11-7/9	MSH2322	(6) 10d	(4) 10d v 1-1/2		1185	840	no further increase a	hall be normit	ind				
14	MSH2322	(6) 10d	(4) 10d v 1-1/2		1185	840	4) 10d x 1-1/2" nails are		.cu. 8″ diameter) by 1-1/2″ lo	na			
16	MSH2322	(6) 10d	(4) 10d x 1-1/2		1185	840	Minimum nail nenetral	tion shall he 1.	-1/2" for 10d nails and 1-		s.		
L."		1-7.00		C″		1	<ul> <li>840 Minimum nail penetration shall be 1-1/2" for 10d nails and 1-5/8 for 16d nails.</li> <li>5) Top Mount Hangers require minimum 3" header width for THO series hangers:</li> </ul>						

- 5) Top Mount Hangers require minimum 3" header width for THO series hangers;
- 3-1/2" minimum header thickness for all other stock numbers.
- 6) Hangers utilizing 16d nails are not compatible with BCI® joists.
- 7) Miter cut required on end of joist to achieve design loads.

8) LSTA24 strap required along top chord for lateral restraint.

- 9) For additional sizes, stock numbers, and modifications not shown,
- 785 refer to USP's Full Line Catalog.

For further information please call 1.800.328.5934 or go to www.USPconnectors.com

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MSH322

MSH322

MSH322

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(6) 10d

(6) 10d

(6) 10d

(6) 10d

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9-1/2

11-7/8

14

16

14 - 20

BCI<sup>®</sup> 90

11-7/8 MSH422

Joist Width = 2-9/16'

(4) 10d x 1-1/2

(4) 10d x 1-1/2

(4) 10d x 1-1/2

Joist Width = 3-1/2

(4) 10d

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1185

1185

1185

1105

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840

840

840

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# DOUBLE I-JOISTS - Canadian/Factored Resistance (lbs)



		Т	op Mount Hangers						Face Mount Ha	ngers		
Joist	USP	Fastene	er Schedule <sup>4,8</sup>	SPF Uplift <sup>3</sup>	Dow	/n 100%²	USP	Fastener	Schedule <sup>4,8</sup>	SPF Uplift <sup>3</sup>	Dow	n 100%²
Height	Stock No.1	Header	Joist	115%	DF	SPF	Stock No.1	Header	Joist	115%	DF	SPF
Doubl	e BCI <sup>®</sup> 5000				Jo	oist Width	= 4″					
9-1/2	THO20950-2	(10) 16d	(6) 10d	1570	4370	3100	THF20925-2	(12) 10d	(6) 10d	2361	5255	3731
11-7/8	THO20118-2	(10) 16d	(6) 10d	1570	4820	3425	THF20112-2	(16) 10d	(6) 10d	2361	6855	4867
14	THO20140-2	(12) 16d	(6) 10d	1570	6060	4305	THF20140-2	(20) 10d	(6) 10d	2361	6735	4782
Doubl	e BCI <sup>∞</sup> 60/6000				Jois	st Width =	4-5/8″					
9-1/2	THO23950-2	(10) 16d	(6) 10d	1570	6005	4265	THF23925-2	(14) 10d	(6) 10d	2361	5240	3720
11-7/8	THO23118-2	(10) 16d	(6) 10d	1570	6005	4265	THF23118-2	(16) 10d	(6) 10d	2361	6855	4867
14	THO23140-2	(12) 16d	(6) 10d	1570	7290	5175	THF23140-2	(20) 10d	(6) 10d	2361	6680	4743
16	THO23160-2	(12) 16d	(6) 10d	1570	7290	5175	THF23160-2	(24) 10d	(6) 10d	2361	6680	4743
Doubl	e BCl <sup>‴</sup> 6500	. ,			Jois	st Width =	5-1/8″	,	.,			
9-1/2	THO25950-2	(10) 16d	(6) 10d	1570	6005	4265	THF25925-2	(12) 10d	(6) 10d	2361	5240	3720
11-7/8	THO25118-2	(10) 16d	(6) 10d	1570	6005	4265	THF25112-2	(16) 10d	(6) 10d	2361	5240	3720
14	THO25140-2	(12) 16d	(6) 10d	1570	6645	4715	THF25140-2	(20) 10d	(6) 10d	2361	6680	4743
16	THO25160-2	(12) 16d	(6) 10d	1570	6645	4715	THF25160-2	(24) 10d	(6) 10d	2361	6680	4743
Doubl	e BCl <sup>®</sup> 90	( )	(1)		Je	hist Width	= 7″	( )	(1)			
11-7/8	BPH71118	(10) 16d	(6) 10d	885	5055	4725	HD7100	(12) 16d	(6) 10d	3149	7215	5123
14	BPH7114	(10) 16d	(6) 10d	885	5055	4725	HD7100	(12) 16d	(6) 10d	3149	7215	5123
16	BPH7116	(10) 16d	(6) 10d	885	5055	4725	HD7140	(20) 16d	(8) 10d	3149	7215	5123
18	BPH7118	(10) 16d	(6) 10d	885	5055	4725	HD7140	(20) 16d	(8) 10d	3149	7215	5123
20	BPH7120	(10) 16d	(6) 10d	885	5055	4725	HD7140	(20) 16d	(8) 10d	3149	7215	5123
20	51111120	(10) IOU	wed 45° Hangers		0000		11BTT10	Ad	iustable Height I	Hangers	1210	0120
Inint	LISP	Eastene	r Schedule <sup>4,8</sup>	SPE Uplift <sup>3</sup>	Dow	/n 100% <sup>2</sup>	USP	Eastener	Schedule <sup>4,8</sup>	SPF Upliff <sup>3</sup>	Dow	n 100%²
JOIST Height	Stock No. <sup>1,6</sup>	llaadan	la la la la la	4459/			Stock No. 1,9	Header	laiat	4459/	DE	
		Header	JOIST	115%	DF	SPF		Header	Joist	115%	DF	SPF
BCI <sup>®</sup> 5		(1.1) 10.1	(10) 10 1	0500	Jo	bist width	= 4		Joist Width =	4		
9-1/2	SKH2020L/R-2'	(14) 10d	(10) 10d	2530	5430	3855						
11-7/8	SKH2020L/R-2*	(14) 10d	(10) 10d	2530	5430	3855						
14	SKH2024L/R-2	(16) 10d	(10) 10d	2530	5055	3590						
Doub	e BCI <sup>®</sup> 60/6000				Jois	st width =	4-5/8	Jo	oist width = 4-	·5/8		
9-1/2	SKH2320L/R-2	(14) 10d	(10) 10d	2530	5430	3855	MSH2322-2°	(6) 10d	(4) 10d		1490	1055
11-7/8	SKH2320L/R-2	(14) 10d	(10) 10d	2530	5430	3855	MSH2322-2°	(6) 10d	(4) 10d		1490	1055
14	SKH2324L/R-2	(16) 10d	(10) 10d	2530	5055	3590	MSH2322-2°	(6) 10d	(4) 10d		1490	1055
Doubl	SKH2324L/K-2 ■ BCI <sup>®</sup> 6500	(10) 100	(10) 100	2530	0000	t Width =	5-1/8″		ist Width = 5-			
9-1/2	SKH2520L/R-27	(14) 10d	(10) 10d	2530	5430	3855	MSH2622-2 <sup>6</sup>	(6) 10d	(4) 10d		1490	1055
11-7/8	SKH2520L/R-27	(14) 10d	(10) 10d	2530	5430	3855	MSH2622-2 <sup>6</sup>	(6) 10d	(4) 10d		1490	1055
14	SKH2524L/R-27	(16) 10d	(10) 10d	2530	5055	3590	MSH2622-2 <sup>6</sup>	(6) 10d	(4) 10d		1490	1055
16	SKH2524L/R-27	(16) 10d	(10) 10d	2530	5055	3590						
Doubl	e BCI <sup>®</sup> 90				J	oist Width	= 7″		Joist Width =	7″		
11-7/8	HD7100-SK45L/R <sup>6,7</sup>	(12) 16d	(6) 10d	2361	7215	5123	MSH422-2	(8) 16d	(6) 16d		2295	1630
14	HD7100-SK45L/R6,7	(12) 16d	(6) 10d	2361	7215	5123						
16	HD7140-SK45L/R6,7	(20) 16d	(8) 10d	2361	7215	5123						
18	HD7140-SK45L/R <sup>6,7</sup>	(20) 16d	(8) 10d	2361	7215	5123						
20	HD7140-SK45L/R6,7	(20) 16d	(8) 10d	2361	7215	5123						
		Varia	ble Pitch Connect	ors			1) Shadad bang	lore roquiro woł	stiffonors at joist	onds Woh stiffs	nore may	ha
		- Eastern	ar Sahadula <sup>4</sup>	SDE Liniift <sup>3</sup>	Dow	m 100% <sup>2</sup>	i) Shaueu hang	lers require wer	Sumeriers at joist	enus. web sune	ners may	De
Joist	USP Otrada No. 19	Fasterie		SPP Oplin	Dow	1 100 %	required for n	on-shaded han	gers by I-joist mar	nufacturers.		
Wdith	Stock No."	Header	Joist	115%	DF	SPF	2) Factored resi	stance is based	I on hanger attach	ment to a DF-L	or S-P-F s	pecies
BCI <sup>®</sup> 5	000						solid sawn h	eader. Some lo	ads may be incre	ased for duration	of load a	djustments
2	TMP21	(6) 10d	(4) 10d x 1-1/2	275	1860	1860	Refer to USI	P Full Line Cata	alog for details.			
	TMPH21	(10) 10d	(8) 10d x 1-1/2	215	3485	2995	<ol><li>Uplift loads had</li></ol>	ave been increa	ased 15% for wind	l and seismic loa	ding; no f	urther
BCI® 6	6000						increase sha	all be permitted.				
2-5/16"	TMP23	(6) 10d	(4) 10d x 1-1/2	275	2570	2570	4) Minimum nail	penetration sha	all be 1-1/2" for 10	d nails and 1-5/8	3" for 16d	nails.
	TMPH23	(10) 10d	(8) 10d x 1-1/2	215	3485	2995	16d sinkers (0	0.148" diameter	) by 3-1/4″ long m	ay be substitute	d for 10d o	common
BCI <sup>®</sup> 6	500						nails with no	load reduction				
2-9/16"	TMP25	(6) 10d	(4) 10d x 1-1/2	275	2630	2630	5) Top Mount H	angers require	minimum 3" heade	er width for THO	series ha	ngers;
2 3/10	TMPH25	(10) 10d	(8) 10d x 1-1/2	215	3485	2995	3-1/2" minimu	im header thick	ness for all other	stock numbers.		
BCI <sup>®</sup> 9	0						6) Hangers are	special order. C	onsult USP for pr	icing and lead tir	nes.	

- 7) Miter cut required on end of joist to achieve design loads.
- 8) Hangers utilizing 16d nails are not compatible with BCI® joists.
- For additional sizes, stock numbers, and modifications not shown, refer to USP's Full Line Catalog.

for non-shaded hangers by I-joist manufacturers. 2) Factored resistance is based on hanger attachment to a DF-L or S-P-F species

(6) 10d

(10) 10d

1) Shaded hangers require web stiffeners at joist ends. Web stiffeners may be required

(4) 10d x 1-1/2

(8) 10d x 1-1/2

275

215

2835

3485

2835

2995

solid sawn. Some loads may be increased for duration of load adjustments. Refer to USP Full Line Catalog for details.

 Uplift loads have been increased 15% for wind and seismic loading; no further increase shall be permitted.

 4) 10d x 1-1/2" nails are 9 gauge (0.148" diameter) by 1-1/2" long. Minimum nail penetration shall be 1-1/2" for 10d nails. For further information please call 1.800.328.5934 or go to www.USPconnectors.com

TMP4

TMPH4

3-1/2"

# Framing Connectors - Simpson Strong-Tie

# SINGLE I-JOISTS - Canadian/Factored Resistance (lbs)

SIMPSON Strong-Tie

			Тор	) Flange						Snap	-In						Fa	ce Mount			
Joist Height	Madal	В	Fast	tener Type	Uplift	Down	Load	<b>N</b>	В	Fastene	r Type	Uplift	Down	Load	<b>N</b>	В	Fast	ener Type	Uplift	Down	Load
neigin	Wodei	Dim	Header	Joist	(115)	DF	SPF	wodei	Dim	Header	Joist	(115)	DF	SPF	wodel	Dim	Header	Joist	Uplift           100           100           270           270           100           270           100           100           270           100           100           100           100           100           100           100           100           100           100           100           100           100           100           100           375           375           375	DF	SPF
BCI 4	500									Joist Width	1 = 1¾"										
91⁄2	LT179	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS1.81/9.5	2	8-10d	_	145	2385	1700	LF179	2	10-10d	1-#8x11⁄4WS	100	2525	2155
117⁄8	LT171188	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS1.81/11.88	2	10-10d	_	145	2565	1835	LF1711	2	12-10d	1-#8x11⁄4WS	100	2840	2155
14	LT1714	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS1.81/14	2	12-10d	_	145	2565	1835	LF1714	2	14-10d	1-#8x11⁄4WS	100	2840	2155
16	LT1716	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS1.81/16	2	14-10d	_	145	2725	1950	MIU1.81/16	21⁄2	24-16d	2-10dx1½	270	3555	2690
BCI 5	000²									Joist Width	1 = 2"										
91⁄2	ITS2.06/9.5	2	6-10d	—	175	2235	1690	IUS2.06/9.5	2	8-10d	_	145	2385	1700			Fastemer Type         Uplitt         Down           Header         Joist         (115)         DF           10-10d         1-#8x11/4WS         100         2525           12-10d         1-#8x11/4WS         100         2840           14-10d         1-#8x11/4WS         100         2840           24-16d         2-10dx11/2         270         3555           0 LF Hanger for these sizes         555         555           10-10d         1-#8x11/4WS         100         2826           12-10d         1-#8x11/4WS         100         2525           12-10d         1-#8x11/4WS         100         2830           14-10d         1-#8x11/4WS         100         2830           14-10d         1-#8x11/4WS         100         2525           12-10d         1-#8x11/4WS         100         2525           12-10d         1-#8x11/4WS         100         3235           24-16d         2-10dx11/2         375         4930           14-10d         1-#8x11/4WS         100         3235           24-16d         2-10dx11/2         375         4930           12-10d         2-#8x11/4WS         100         3235				
111%	ITS2.06/11.88	2	6-10d	_	175	2235	1690	IUS2.06/11.88	2	10-10d	_	145	2565	1835		ī	Vo LF Hang	ger for these size	es		
14	ITS2.06/14	2	6-10d	—	175	2235	1690	IUS2.06/14	2	12-10d	_	145	2565	1835							
BCI 6	0/6000²									Joist Widtl	1 = <b>2</b> 5⁄16"										
91⁄2	LT239	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS2.37/9.5	2	8-10d	—	145	2385	1700	LF239	2	10-10d	1-#8x1¼WS	100	2525	2155
111%	LT231188	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS2.37/11.88	2	10-10d	—	145	2565	1835	LF2311	2	12-10d	1-#8x1¼WS	100	2880	2270
14	LT2314	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS2.37/14	2	12-10d	—	145	2565	1835	LF2314	2	14-10d	1-#8x1¼WS	100	3235	2380
16	LT2316	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS2.37/16	2	14-10d	—	145	2725	1950	MIU2.37/16	21⁄2	24-16d	2-10dx1½	375	4930	3485
BCI 6	500²									Joist Widtl	h = 2%16"	1							Joist         (115)           #8x11/4WS         100           #8x11/4WS		
91⁄2	LT259	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS2.56/9.5	2	8-10d	—	145	2385	1700	LF259	2	10-10d	1-#8x11⁄4WS	100	2525	2155
117⁄8	LT251188	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS2.56/11.88	2	10-10d	—	145	2565	1835	LF2511	2	12-10d	1-#8x11⁄4WS	100	2880	2270
14	LT2514	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS2.56/14	2	12-10d	—	145	2565	1835	LF2514	2	14-10d	1-#8x11/4WS	100	3235	2380
16	LT2516	2	6-10d	1-#8x11⁄4WS	100	2620	1725	IUS2.56/16	2	14-10d	—	145	2725	1950	MIU2.56/16	21⁄2	24-16d	2-10dx1½	375	4930	3485
BCI 9	0									Joist Width	1 = 31⁄2"										
117⁄8	LT351188	2	6-10d	2-#8x11/4WS	100	2620	1725	IUS3.56/11.88	2	12-10d	_	145	2375	1695	LF3511	2	12-10d	2-#8x11⁄4WS	100	2880	2270
14	LT3514	2	6-10d	2-#8x11/4WS	100	2620	1725	IUS3.56/14	2	12-10d	_	145	2375	1695	LF3514	2	14-10d	2-#8x11⁄4WS	100	3235	2380
16	LT3516	2	6-10d	2-#8x11/4WS	100	2620	1725	IUS3.56/16	2	14-10d	_	145	2375	1695	MIU3.56/16	21⁄2	24-16d	2-10dx11/2	375	4930	3480
18	MIT418	21⁄2	8-16d	2-10dx11/2	535	3480	2415			S Hanger f	or these	eizae			MIU3.56/18	21⁄2	26-16d	2-10dx11/2	375	4930	3480
20	MIT420	21⁄2	8-16d	2-10dx11/2	535	3480	2415		10 10	o nanyel i	or litese	31283			MIU3.56/20	21⁄2	28-16d	2-10dx1½	375	4930	3480

Shaded hangers require web stiffeners at joist ends. Web stiffeners may be required by others for non-shaded hangers.
 At max capacity shown hangers may exceed standard 1/4" deflection by 1/32".

3. THAI hangers require a minimum of 4 top and two face nails installed. 4. The B Dim is the depth of the hanger seat.

			45°	Skew						Adjusta	able Height						Field S	Slope & Skew			
J01St Height	Model	В	Faste	ener Type	Uplift	Dowr	n Load	Madal	В	Faste	ener Type	Uplift	Down	Load	Madal	В	Fast	ener Type	Uplift	Down	ı Load
norgin	Wouer	Dim	Header	Joist	(115)	DF	SPF	Wouei	Dim	Header	Joist	(115)	DF	SPF	wouer	Dim	Header	Joist	(115)	DF	SPF
BCI 4	4500									Joist Wi	idth = 1¾"										
91⁄2	SUR/L1.81/9	33⁄16	14-16d	2-10dx11/2	275	3140	2220	THAI1.81/22	21⁄4	6-10d	2-10dx11/2	_	2740	2075	LSSUI25	31⁄2	9-10d	7-10dx1½	1285	2090	1495
117⁄8	SUR/L1.81/11	33⁄16	16-16d	2-10dx11⁄2	275	3140	2220	THAI1.81/22	21⁄4	6-10d	2-10dx1½	—	2740	2075	LSSUI25	3½	9-10d	7-10dx1½	1285	2090	1495
14	SUR/L1.81/14	33⁄16	18-16d	2-10dx11/2	275	3140	2220	THAI1.81/22	21⁄4	6-10d	2-10dx11/2	—	2740	2075	LSSUI25	31⁄2	9-10d	7-10dx1½	1285	2090	1495
16	SUR/L1.81/14	3¾16	18-16d	2-10dx1½	275	3140	2220	See Canadian V	Vood Co	nstruction C	<i>onnectors</i> catalo	gue for ha	inger sele	ction.	See Canadian	Wood C	Construction	Connectors catalo	gue for ha	anger sel	ection.
BCI 5	5000²									Joist Wi	idth = 2"										
91⁄2	SUR/L2.06/9	33⁄16	14-16d	2-10dx11/2	385	3945	2780	THAI2.06/22	21⁄4	6-10d	2-10dx11/2	—	2740	2075	LSSUI2.06	31⁄2	9-10d	7-10dx1½	1285	2090	1495
117⁄8	SUR/L2.06/11	33⁄16	16-16d	2-10dx11⁄2	385	3945	2780	THAI2.06/22	21⁄4	6-10d	2-10dx1½	—	2740	2075	LSSUI2.06	3½	9-10d	7-10dx1½	1285	2090	1495
14	SUR/L2.06/11	33⁄16	16-16d	2-10dx11/2	385	3945	2780	THAI2.06/22	21⁄4	6-10d	2-10dx11/2	_	2740	2075	LSSUI2.06	31⁄2	9-10d	7-10dx1½	1285	2090	1495
BCI 6	6 <b>0/6000</b> ²									Joist W	idth = 25⁄16"										
91⁄2	SUR/L2.37/9	33⁄16	14-16d	2-10dx11/2	385	3945	2780	THAI3522	21⁄4	6-10d	2-10dx11/2	_	2740	2075	LSSUI35	31⁄2	9-10d	7-10dx1½	1285	2090	1495
117⁄8	SUR/L2.37/11	33⁄16	16-16d	2-10dx11⁄2	385	3945	2780	THAI3522	21⁄4	6-10d	2-10dx1½	—	2740	2075	LSSUI35	3½	9-10d	7-10dx1½	1285	2090	1495
14	SUR/L2.37/14	33⁄16	18-16d	2-10dx11/2	385	3945	2780	THAI3522	21⁄4	6-10d	2-10dx11/2	—	2740	2075	LSSUI35	31⁄2	9-10d	7-10dx1½	1285	2090	1495
16	SUR/L2.37/14	33⁄16	18-16d	2-10dx11/2	385	3945	2780	See Canadian V	Vood Co	nstruction C	onnectors catalo	gue for ha	inger sele	ction.	See Canadian	Wood C	Construction	Connectors catalo	gue for ha	anger sel	ection.
BCI 6	6 <b>500</b> ²									Joist Wi	idth = 2%16"										
91⁄2	SUR/L2.56/9	3¾16	14-16d	2-10dx11/2	385	3945	2780	THAI322	21⁄4	6-10d	2-10dx11/2	—	2740	2075	LSSUH310	31⁄2	14-16d	12-10dx1½	1725	2620	1850
117⁄8	SUR/L2.56/11	33⁄16	16-16d	2-10dx11/2	385	3945	2780	THAI322	21⁄4	6-10d	2-10dx11/2	—	2740	2075	LSSUH310	31⁄2	14-16d	12-10dx1½	1725	2620	1850
14	SUR/L2.56/14	33⁄16	18-16d	2-10dx11/2	385	3945	2780	THAI322	21⁄4	6-10d	2-10dx11/2	—	2740	2075	LSSUH310	31⁄2	14-16d	12-10dx1½	1725	2620	1850
16	SUR/L2.56/14	33⁄16	18-16d	2-10dx11/2	385	3945	2780	See Canadian V	Vood Co	nstruction C	<i>onnectors</i> catalo	gue for ha	inger sele	ction.	See Canadian	Wood C	Construction	Connectors catalo	gue for ha	anger sel	ection.
BCI 9	90									Joist Wi	idth = 3½"										
117⁄8	SUR/L410	25⁄8	14-16d	6-16d	1975	4065	2875	THAI422	21⁄4	6-10d	2-10dx1½	—	2740	2075	LSSU410	3½	14-16d	12-10dx1½	1725	3055	2160
14	SUR/L414	21⁄2	18-16d	8-16d	2175	4095	2895	THAI422	21⁄4	6-10d	2-10dx11/2	_	2740	2075	LSSU410	3½	14-16d	12-10dx1½	1725	3055	2160
16	SUR/L414	21⁄2	18-16d	8-16d	2175	4095	2895					_				_			_		
18	SUR/L414	21⁄2	18-16d	8-16d	2175	4095	2895	See	Canad cat	ian <i>Wood</i> aloque for	Construction hanger select	<i>Connect</i> ion.	ors		S	e Cana c:	idian <i>Wool</i> atalogue fo	d Construction Ir hanger select	Connect tion.	tors	
20	SUR/L414	21/2	18-16d	8-16d	2175	4095	2895		out							0	alaloguo lu				
estern S	Specifier Guide -	CANA	DA																		Mar

# **DOUBLE I-JOISTS – Canadian/Factored Resistance (Ibs)**

SIMPSON Strong-Tie

25

	Tan Elanno Enere Mount											© ∕15° Skow									
Joist		-	101	-range	IL PE		Local		-	Fac	e-mount	11.00	D				40	OKCW	Harre	<b>D</b>	Lood
Height	Model	B	Fasi	ener Type		DOWN	LOad	Model	B	Faste	ner Type		DOWI	LOad	Model	B	Faste	ener Type		Down	LOad
		DIN	Header	Joist	(115)	DF	SPF		Dilli	Header	Joist	(115)	DF	SPF		DIM	Header	Joist	(115)	DF	SPF
Dout	IE BCI 4500				505	0.400				Joist W	$10th = 3\frac{1}{2}$ "	075	1550	0045		0.000			1075	5070	0700
91/2	MI149.5	21/2	8-16d	2-10dx1½	535	3480	2415	MIU3.56/9	21/2	16-16d	2-10dx1½	375	4550	3215	HSUR/L410	21/16	20-16d	6-16d	1975	5270	3730
111/8	MI1411.88	21/2	8-16d	2-10dx1½	535	3480	2415	MIU3.56/11	21/2	20-16d	2-10dx1½	375	4550	3215	HSUR/L410	21/16	20-16d	6-16d	1975	5270	3730
14	MI1414	21/2	8-16d	2-10dx1½	535	3480	2415	MIU3.56/14	21/2	22-16d	2-10dx1½	375	4930	3485	HSUR/L414	21/16	26-16d	8-16d	2615	6880	4665
16	MI1416	21/2	8-16d	2-10dx1½	535	3480	2415	MIU3.56/16	21/2	24-16d	2-10dx1½	375	4930	3485	HSUR/L414	21/16	26-16d	8-16d	2615	6880	4665
Dout	le BCI 5000 <sup>3</sup>		T	1	1					Joist W	idth = 4"			1				1			
91⁄2	MIT4.12/9.5	21/2	8-16d	2-10dx1½	535	3480	2415	MIU4.12/9	21/2	16-16d	2-10dx1½	375	4550	3215	HSUR/L4.12/9	3	12-16d	2-10dx1½	275	2995	2350
117/8	MIT4.12/11.88	3 21/2	8-16d	2-10dx1½	535	3480	2415	MIU4.12/11	21/2	20-16d	2-10dx1½	375	4550	3215	HSUR/L4.12/11	3	16-16d	2-10dx1½	275	4195	2965
14	MIT4.12/14	21/2	8-16d	2-10dx1½	535	3480	2415	MIU4.12/14	21/2	22-16d	2-10dx1½	375	4930	3485	HSUR/L4.12/14	3	20-16d	2-10dx1½	275	4195	2965
Dout	le BCI 60	- 1	Т		1	1	1	1	1	Joist W	idth = 4%"	1	1	1	1	-	1	1	1	1	
117⁄8	MIT3511.88-2	21/2	8-16d	2-10dx1½	535	3480	2415	MIU4.75/11	21⁄2	20-16d	2-10dx1½	375	4550	3215	HSUR/L4.75/11	23/4	16-16d	2-10dx11/2	275	4195	2965
14	MIT3514-2	21/2	8-16d	2-10dx1½	535	3480	2415	MIU4.75/14	21⁄2	22-16d	2-10dx11/2	375	4930	3485	HSUR/L4.75/14	23/4	20-16d	2-10dx1½	275	4195	2965
16	MIT4.75/16	21/2	8-16d	2-10dx11/2	535	3480	2415	MIU4.75/16	21⁄2	24-16d	2-10dx11/2	375	4930	3485	HSUR/L4.75/16	23⁄4	24-16d	2-10dx11/2	275	4195	2965
Dout	le BCI 6000 <sup>3</sup>		-			T	1	•	1	Joist W	idth = 45⁄8"	1	1	-	-					1	
91⁄2	MIT359.5-2	21/2	8-16d	2-10dx11/2	535	3480	2415	MIU4.75/9	21⁄2	16-16d	2-10dx11/2	375	4550	3215	HSUR/L4.75/9	23/4	12-16d	2-10dx11/2	275	2995	2350
111/8	MIT3511.88-2	21/2	8-16d	2-10dx11/2	535	3480	2415	MIU4.75/11	21⁄2	20-16d	2-10dx11/2	375	4550	3215	HSUR/L4.75/11	23/4	16-16d	2-10dx11/2	275	4195	2965
14	MIT3514-2	21/2	8-16d	2-10dx11/2	535	3480	2415	MIU4.75/14	21⁄2	22-16d	2-10dx11/2	375	4930	3485	HSUR/L4.75/14	23/4	20-16d	2-10dx11/2	275	4195	2965
16	MIT4.75/16	21/2	8-16d	2-10dx11/2	535	3480	2415	MIU4.75/16	21⁄2	24-16d	2-10dx11/2	375	4930	3485	HSUR/L4.75/16	23⁄4	24-16d	2-10dx11/2	275	4195	2965
Dout	le BCI 6500 <sup>3</sup>									Joist W	'idth = 51⁄8"										
91⁄2	MIT39.5-2	21/2	8-16d	2-10dx11/2	535	3480	2415	MIU5.12/9	21⁄2	16-16d	2-10dx11/2	375	4550	3215	HSUR/L5.12/9	2 <sup>13</sup> ⁄16	12-16d	2-10dx11/2	275	2995	2350
111/8	MIT311.88-2	21/2	8-16d	2-10dx11/2	535	3480	2415	MIU5.12/11	21⁄2	20-16d	2-10dx11/2	375	4550	3215	HSUR/L5.12/11	2 <sup>13</sup> ⁄16	16-16d	2-10dx11/2	275	4195	2965
14	MIT314-2	21/2	8-16d	2-10dx1½	535	3480	2415	MIU5.12/14	21⁄2	22-16d	2-10dx11/2	375	4930	3485	HSUR/L5.12/14	2 <sup>13</sup> /16	20-16d	2-10dx11/2	275	4195	2965
16	MIT5.12/16	21/2	8-16d	2-10dx11/2	535	3480	2415	MIU5.12/16	21/2	24-16d	2-10dx11/2	375	4930	3485	HSUR/L5.12/16	213/16	24-16d	2-10dx11/2	275	4195	2965
Dout	le BCI 90									Joist W	idth = 7"		l				1				
11%	B7.12/11.88	21/2	14-16d	6-16d	1650	5940	3910	HU412-2	21/2	22-16d	8-16d	2635	5780	4670	HU412-2X <sup>2</sup>	21/2	22-16d	8-16d	1975	3775	3035
14	B7.12/14	21/2	14-16d	6-16d	1650	5940	3910	HU414-2	21/2	26-16d	12-16d	3800	7025	5780	HU414-2X <sup>2</sup>	21/2	26-16d	12-16d	2850	4565	3755
16	B7.12/16	21/2	14-16d	6-16d	1650	5940	3910	HU414-2	21/2	26-16d	12-16d	3800	7025	5780	HU414-2X <sup>2</sup>	21/2	26-16d	12-16d	2850	4565	3755
18	B7.12/18	21/2	14-16d	6-16d	1650	5940	3910	HU414-2	21/2	26-16d	12-16d	3800	7025	5780	Se	e Canadi	an <i>Wood (</i>	Construction (	Connecto	ors	
20	B7.12/20	21/	14-16d	6-16d	1650	5940	3910	See Canadian V	Vood Co	nstruction C	onnectors catalo	aue for h	anger sel	lection.	-	cata	logue for l	hanger selecti	on.		
joist by ot	ends. Web s hers for nor	stiffenei 1-shade	s may be d hangers Adjusta	required S. able Height	Spe (e.g	cify ski . HU41	ew ang 2-2x, S	le and directi SKR45°).	on	hi 1/1 Field	angers may " deflection Slope & Skev	exceed by 1/32 v	d stand ".	dard	31/8" and 5. LSU's ar ordered.	5%16". e field :	sloped or	nly. Skew op	ition m	ust be	factory
Joist		в	Fastene	er Type	Unlift	Down	Load		ь	Fas	tener Tvpe	lin	liff	Down L	oad						
Height	Model	Dim	leader	loist	(115)	DE	SPF	Model	Dim	Header	Inist	(1	15)	DE		то		A HANG	ER:		
Doub	In DOL 4500		louuol	00101	. ,	Di la	int Mid	IL 01/1		Houdor	00101	·	<i>,</i>								
Doub	IE BUI 4000					JO	ist wid	ln = 3 ½"		1	1	-	-		1. Find	your jo	oist size	in this guid	e.		
91⁄2	THAI422	21⁄2	6-10d	2-10dx1½	—	2740	2075	LSSU410	31/2	14-16d	12-10dx11	<sup>2</sup> 17	25 3	055 2	2160 2 Cho	nse vo	ur heada	ertvne Sol	id hear	der or	l-inist
111 //8	THAI422	21⁄2	6-10d	2-10dx1½	—	2740	2075	LSSU410	31⁄2	14-16d	12-10dx11	⁄2 17	25 3	055 2	2160	000 90		, type: eei	ia noa		i joiot.
14	THAI422	21⁄2	6-10d	2-10dx1½	_	2740	2075	LSSU410	31/2	14-16d	12-10dx11	⁄2 17	25 3	055 2	2160	Solid h	eaders i	nclude soli	d sawr	n Dou	glas Fir,
16	See Canadia	n Wood C	onstruction C	onnectors catalog	ue for ha	nger sele	ction.	See Canadiar	n Wood i	Construction	Connectors cata	alogue for	r hanger :	selection		Spruce	e-Pine F	26-16d       8-16d       2615       6880       4         26-16d       8-16d       2615       6880       4         22-16d       2-10dx1½       275       2995       2         16-16d       2-10dx1½       275       4195       2         20-16d       2-10dx1½       275       4195       2         16-16d       2-10dx1½       275       4195       2         12-16d       2-10dx1½       275       4195       2         16-16d       2-10dx1½       275       4195       2         12-16d       8-16d       1975       3775       3         16-16d       12-16d       2850       4565       3         12-16d       8-16d       1975       3775       3 </td			
Doub						-						-				For Lic	hist head	lor soo naa	<u>     8                               </u>		
01/		01/	6 101	0.104.11/		0005	0005	1 0114 105	01/	04.464	10 10 4 11	( 10		705 (	0005	i oi i-je	13111040	ici see pag	0.0.		
91/2	THAI-2"	Z 1/2	6-100 i	2-100X11/2	_	2935	2935	LSU4.12 <sup>3</sup>	31/2	24-160	16-100X19	2 19	160 3	700 4	3. Loca	ate you	r conneo	ctor type in	the tal	ble.	
117⁄8	(HAI-24	21⁄2	6-10d :	2-10dx1½	_	2935	2935	LSU4.12 <sup>5</sup>	31/2	24-16d	16-10dx11	<sup>/</sup> 2 19	60 3	765 2	2665	Facor	nount to	n flango ol	COMON	elono	d ata
14	THAI-24	21⁄2	6-10d	2-10dx1½	—	2935	2935	LSU4.125	31/2	24-16d	16-10dx11	⁄2 19	60 3	765	2665	1 aue 11	iouni, i0	p nange, Si	.eweu	, siope	u, eic.
Doub	le BCI 60/600	<b>10</b> 3				Jo	ist Wid	th = 4%"							4. Sele	ct a ha	inger fro	m the table			
91/2	THAI-24	21/2	6-10d	2-10dx1½	_	2935	2935	LSU3510.2 <sup>5</sup>	31/2	24-16d	16-10dx11	2 19	60 3	765	2665	C ()			1	taa te	1
117/6	THAI-24	21/2	6-10d	2-10dx11/	_	2035	2935	LSU3510.25	31/	24-164	16-10dv11	6 10	160 2	765	5. Con	urm tha	at your ta	actored jois	i react	ION IS	iess tha
4.4		01/	6 10-1	0 104-41/		2000	2000	1010540.05	0/2	04.40.	10 100 11	/		765		actore		tod your bo	iyel. II naor	yes, y	
14	THAI-2"	Z1⁄2	0-100	2-100X11/2	_	2935	2935	LSU3510.2°	31/2	24-16d	10-10dx11	2 19	3 טטי	0/05	SUC	เธรรเป	ny selec	ieu your na	ngel.		
16	See Canadia	n <i>Wood C</i>	onstruction C	onnectors catalog	ue for ha	nger sele	ction.	See Canadiar	n <i>Wood</i> i	Construction	Connectors cata	alogue foi	r hanger :	selection	. If yo	u did n	ot find a	euitabla ba	naor	nloca	soo th
Doub	le BCI 6500 <sup>3</sup>					Jo	ist Wid	th = 51⁄8"							ii yo	u uiù lì Cont Co	ot IIIIU a		uiyer; tructio		beetors
91⁄2	THAI-24	21⁄2	6-10d	2-10dx1½	_	2935	2935	LSU5.125	31/2	24-16d	16-10dx13	2 12	85 2	600	1835 oct		or call C	impeon Ctr		11 UUI	001 000
117%	THAI-24	21/2	6-10d	2-10dx11/3	_	2935	2935	LSU5 125	31/4	24-16d	16-10dv11	6 19	85 2	600	1835 500	aogue 9	u call S	11102011 311	ung-11	e al (C	00) 999
14	THAL-24	21/-	6-104	2-10dv11/		2025	2025	1 \$115 125	01/	2/ 164	16-10dv11	6 10	85 0	600	1835	<b>.</b>					
14	THAI-2'	∠ 1⁄2	u-100	2-1UUX11/2	_	2900	∠ສວວ	1303.123	31/2	24-100	10-100x15	2 12	.00 2	.000	You	will ne	ed the fo	ollowing info	ormatio	on:	
16	16 See Canadian Wood Construction Connectors catalogue for hanger selection. See Canadian Wood Construction Connectors catalogue for hanger selection.									• Do	wnload	1									
Doub	le BCI 90					Jo	ist Wid	th = 7"							• Up	lift					
117⁄8															•He	ader co	ondition	autaz			
14															• Be	aring le	ength ree	quirement			
16	S	ee Cana	dian <i>Wood</i>	Construction (	Connect	ors		S	ee Can	adian <i>Woo</i>	d Constructio	n Conn	ectors								
18		Ca	talogue for	hanger selecti	on.				C	atalogue f	or hanger sele	ection.									
10	1														1						

20

### **General Notes**

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- See current Canadian Wood Construction Connectors catalogue for Important Information and General Notes section and for hanger models, joist sizes, and header situations not shown. See the Simpson Strong-Tie Connector Guide CSG-BCCANBCI10 1/12 for installation information.
- Unless otherwise noted, factored 2. resistances (downloads) listed address hanger/header/fastener limitations assuming header material is Douglas Fir-Larch or Spruce Pine Fir. For LVL headers made primarily of Douglas Fir/Southern Pine, use the values found in the DF column. For LVL headers made primarily from Spruce Pine Fir or similar less dense veneers, use the values found in the SPF column. Loads are in pounds. Joist reaction should be checked by a qualified designer to ensure proper hanger selection.
- 3. Factored uplift resistances (uplift) listed assume SPF joist and header

and have been increased by 15% for earthquake and wind loading with no further increase allowed. Reduce loads according to code for normal duration loading such as cantilever construction.

- If hanger height is less than 60% of joist height, joist rotation may occur; see information in the Simpson Strong-Tie Connector Guide CSG-BCCANBCI10 1/12.
- Top flange hanger configuration and thickness of top flange need to be considered for flush frame conditions, see the Simpson Strong-Tie Connector Guide CSG-BCCANBCI10 1/12.
- 6. For this publication, carrying members are assumed to be at least 5½ inches tall. The horizontal thickness of the carrying member must be at least the length of nail being used or the hanger top flange dimension, whichever is greater. *Exception: narrower carrying members may be used with face mount hangers but the horizontal thickness*

must be at least 1¾ inches for 10d nails; 2 inches for 16d nails. Clinch nails on back side.

- THAI hangers in this publication are based on a "top flange" installation and require that the carrying member have a horizontal thickness of at least 2<sup>1</sup>/<sub>2</sub> inches. Backer blocks are required when the header is an I-joist.
- 8. All nails shown are common nails unless otherwise noted.
- I-joists that are used as headers require backer blocks. See the Simpson Strong-Tie Connector Guide CSG-BCCANBCI10 1/12, Wood I-Joist Headers below for additional information.
- 10. **Multiple Members:** Multiple members should be adequately connected together to act as one unit.



# Weights of Building Materials

CEILINC Bounda Por Square	Foot (DSE)
CEILING Pounds Per Square	
	1
Suspended steel channel system ()	2
Suspended wood channel system	2.5
2x8 ceiling joists @ 16" o.c., R-49 insulation,	7
1/2" gypsum board	
1" Plaster	8
1/2" gypsum board	2.2
%" gypsum board	2.75
ROOF Pounds Per Square	Foot [PSF]
Fiberglass shingles	3
Asphalt shingles (1)	2
Wood shingles (1)	3
Spanish clav tile (1)	19
Composition Roofing	
Three-ply ready roofing <sup>(1)</sup>	1
Four ply felt and gravel (1)	5.5
Five ply felt and gravel (1)	5.5
20 gago motal dock (1)	2.5
19 gage metal deck (1)	2.5
	3
1" fiberglass batt insulation	0.04
1" loose fiberglass insulation	0.04
1" loose cellulose insulation	0.14
1" rigid insulation <sup>(1)</sup>	1.5
<sup>3</sup> / <sub>16</sub> " slate <sup>(1)</sup>	7
1/4" slate (1)	10
Single-ply (no ballast) (1)	0.7
Single-ply (ballasted)	11
Dry gravel <sup>(1)</sup>	8.7
2x8 rafters @ 16" o.c., fiberglass shingles, 15# felt.	
%" sheathing	8
Skylight: metal frame w/ 3/8" wire glass (1)	8
FLOOR Pounds Per Square	Foot (PSF)
1" reinforced regular weight concrete	12.5
1" plain lightweight concrete (1)	8
7/16" cementitious backerboard	3
Ceramic or quarry tile $(\frac{3}{4})$ on $\frac{1}{2}$ mortar bed $(1)$	16
Ceramic or quarry tile ( <sup>3</sup> / <sub>4</sub> ") on 1" mortar bed <sup>(1)</sup>	23
1" mortar bed	12
1" slate <sup>(1)</sup>	15
<sup>3</sup> / <sub>8</sub> " marble tile	6
<sup>3</sup> / <sub>8</sub> " ceramic floor tile <sup>(1)</sup>	4.7
Hardwood flooring, 7/7-in <sup>(1)</sup>	4
$\frac{1}{4}$ linoleum or asphalt tile <sup>(1)</sup>	1
BCI <sup>®</sup> /AJS <sup>®</sup> joists @ 16" o.c., <sup>3</sup> /" sheathing, <sup>1</sup> //" gypsum board	10
<sup>3</sup> / <sup>4</sup> " Gvp-Crete topping	6.5
Carpet & Pad	2.0
Waterproofing Membranes	
Bituminous smooth surface <sup>(1)</sup>	15
	1
MISCELLANEOUS Pounds Per Square	Foot [PSF]
1" of sand	8
1" of water	52
Hav: baled dry <sup>(2)</sup>	15 DCE(2)
Strow: balod dry (2)	
Sulaw. Daleu, Uly 🖙	
Saturated Soli (garden/landScaped root)	135 PUF
Grand plano	1000 LBS

(1) Minimum Design Loads for Buildings and Other Structures, ASCE 7-05.

(2) National Farm Building Code (Canada) 1995. Value in pounds per cubic foot (PCF), multiply by maximum height to obtain PSF.

SHEATHING	Pounds I	Per	Square	Foot [	PSF1
<sup>11</sup> / <sub>32</sub> " or <sup>3</sup> / <sub>8</sub> " Plywood – OSB <sup>(3)</sup>				1.0	- 1.2
<sup>15</sup> / <sub>32</sub> " or <sup>1</sup> / <sub>2</sub> " Plywood – OSB <sup>(3)</sup>				1.4	- 1.7
<sup>19</sup> / <sub>32</sub> " or <sup>5</sup> / <sub>8</sub> " Plywood – OSB <sup>(3)</sup>				1.8	- 2.1
<sup>23</sup> / <sub>32</sub> " or <sup>3</sup> / <sub>4</sub> " Plywood – OSB <sup>(3)</sup>				2.2	- 2.5
7/8" Plywood – OSB (3)				2.6	- 2.9
1 <sup>1</sup> / <sub>8</sub> " Plywood – OSB <sup>(3)</sup>				3.3	- 3.6
1/2" cementitious backerboard					3
11/2" softwood T & G decking				4	.6
FLOOR FRAMING	Pounds I	Per	Square	Foot [	PSF]
2x4 @ 16" o.c.				1	.1
2x6 @ 16" o.c.				1	.7
2x8 @ 16" o.c.				2	.2
2x10 @ 16" o.c.				2	.9
2x12 @ 16" o.c.				3	.5
BCI® 4500s, 5000 or 5000s @ 12" o.c.				2.1	- 2.9
BCI® 4500s, 5000 or 5000s @ 16" o.c.				1.6	- 2.2
BCI® 4500s, 5000 or 5000s @ 19.2" o.	С.			1.3	– 1.8
BCI® 4500s, 5000 or 5000s @ 24" o.c.				1.1	- 1.5
BCI <sup>®</sup> 6000 or 6000s @ 12" o.c.				2.5	- 3.4
BCI® 6000 or 6000s @ 16" o.c.				1.9	- 2.6
BCI® 6000 or 6000s @19.2" o.c.				1.6	- 2.1
BCI® 6000 or 6000s @ 24" o.c.				1.3	- 1.7
BCI® 60, 60s, 6500 or 6500s @ 12" o.c	).			2.5	- 3.8
BCI® 60, 60s, 6500 or 6500s @ 16" o.c	).			1.9	- 2.9
BCI® 60, 60s, 6000 or 6500s @19.2" o	.C.			1.6	- 2.4
BCI® 60, 60s, 6500 or 6500s @ 24" o.c	).			1.3	– 1.9
BCI <sup>®</sup> 90, 90s or 90e @ 12" o.c.				3.9	- 5.4
BCI <sup>®</sup> 90, 90s or 90e @ 16" o.c.				2.9	- 4.1
BCI <sup>®</sup> 90, 90s or 90e @ 19.2" o.c.				2.4	- 3.4
BCI <sup>®</sup> 90, 90s or 90e @ 24" o.c.				1.9	- 2.7
AJS <sup>®</sup> 140 or 20 @ 12" o.c.				2.2	- 3.3
AJS <sup>®</sup> 140 or 20 @ 16" o.c.				1.7	- 2.5
AJS <sup>®</sup> 140 or 20 @ 19.2" o.c.				1.4	– 2.1
AJS <sup>®</sup> 140 or 20 @ 24" o.c.				1.1	– 1.7
AJS <sup>®</sup> 25 @ 12" o.c.				3.1	- 5.4
AJS <sup>®</sup> 25 @ 16" o.c.				2.3	- 4.1
AJS <sup>®</sup> 25 @ 19.2" o.c.				1.9	- 3.4
AJS <sup>®</sup> 25 @ 24" o.c.				1.6	- 2.7
WALL	Pounds I	Per	Square	Foot [	PSF]
<sup>5</sup> / <sub>16</sub> " x 7 <sup>1</sup> / <sub>2</sub> " fiber cement lap siding					3
4" clay brick <sup>(1)</sup>				3	39
1/4" ceramic wall tile (1)				3	3.1
1 <sup>3</sup> ⁄ <sub>4</sub> " Cultured Stone				1	12
2x4 studs @ 16" o.c., 5%" gypsum, inst	ulation, ¾	8" S	iding <sup>(1)</sup>		11
2x6 studs @ 16" o.c., 5⁄8" gypsum, inst	ulation, ¾	́а" S	iding <sup>(1)</sup>	1	12
Wood or steel studs, 1/2" gypsum bo	oard eac	ch s	side (1)		8
Exterior stud walls w/ brick veneer	(1)			2	18
Stucco				1	10
Log Wall: 10" diameter				2	26
Glass Block:					
4" Thick - standard (hollow)				2	20
3" Thick - standard (hollow)				-	16
4" Thick - thin face				2	30
3" Thick - solid class block				2	10
Windows: glass frame and sash (1)					8
Include at least 1.5 nef in all dead lo	oad eur	m	atione to	າ ຂດດ	ount
monute al least 1.5 psi ili all dedu l	Jau Sull	1110			Jun

(3) Approximate Engineering Dead Load Weight of Wood Structural Panels, APA EWS TT-019, 2005.

# Great products are only the beginning.®

![](_page_27_Picture_1.jpeg)

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